GENERAL NOTES

THESE GENERAL NOTES ARE TO BE USED AS A SUPPLEMENT TO THE SPECIFICATIONS. ANY DISCREPANCIES FOUND AMONG THE DRAWINGS, THE SPECIFICATIONS, THESE GENERAL NOTES AND THE SITE CONDITIONS SHALL BE REPORTED TO THE ARCHITECT, WHO SHALL CORRECT SUCH DISCREPANCY IN WRITING. ANY WORK DONE BY THE GENERAL CONTRACTOR AFTER DISCOVERY OF SUCH DISCREPANCY SHALL BE DONE AT THE GENERAL CONTRACTOR'S RISK. THE GENERAL CONTRACTOR SHALL VERIFY AND COORDINATE DIMENSIONS AMONG ALL DRAWINGS PRIOR TO PROCEEDING WITH ANY WORK OR FABRICATION. THE STRUCTURE HAS BEEN DESIGNED TO RESIST CODE SPECIFIED VERTICAL AND LATERAL FORCES AFTER THE CONSTRUCTION OF ALL STRUCTURAL ELEMENTS HAS BEEN COMPLETED. STABILITY OF THE STRUCTURE PRIOR TO COMPLETION IS THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR. THIS RESPONSIBILITY INCLUDES BUT IS NOT LIMITED TO JOB SITE SAFETY; ERECTION MEANS, METHODS, AND SEQUENCES; TEMPORARY SHORING, FORMWORK, BRACING; USE OF EQUIPMENT AND CONSTRUCTION PROCEDURES. PROVIDE ADEQUATE RESISTANCE TO LOADS ON THE STRUCTURES DURING CONSTRUCTION PER SEI/ASCE STANDARD NO. 37-14 "DESIGN LOADS ON STRUCTURES DURING CONSTRUCTION."

CONSTRUCTION OBSERVATION BY THE STRUCTURAL ENGINEER IS FOR GENERAL CONFORMANCE WITH DESIGN ASPECTS ONLY AND IS NOT INTENDED IN ANY WAY TO REVIEW THE CONTRACTOR'S CONSTRUCTION

ALL METHODS, MATERIALS AND WORKMANSHIP SHALL CONFORM TO THE 2015 INTERNATIONAL BUILDING CODE (IBC) AS AMENDED AND ADOPTED BY THE LOCAL BUILDING OFFICIAL OR APPLICABLE JURISDICTION.

CONTRACT DRAWINGS / DIMENSIONS

ARCHITECTURAL DRAWINGS ARE THE PRIME CONTRACT DRAWINGS. CONSULTANT DRAWINGS BY OTHER DISCIPLINES ARE SUPPLEMENTARY TO ARCHITECTURAL DRAWINGS. REPORT DIMENSIONAL OMISSIONS OR DISCREPANCIES BETWEEN ARCHITECTURAL DRAWINGS AND STRUCTURAL, MECHANICAL, ELECTRICAL OR CIVIL DRAWINGS TO ARCHITECT PRIOR TO PROCEEDING WITH WORK.

STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH ARCHITECTURAL DRAWINGS. PRIMARY STRUCTURAL ELEMENTS ARE DIMENSIONED ON STRUCTURAL PLANS AND DETAILS AND OVERALL LAYOUT OF STRUCTURAL PORTION OF WORK. SOME SECONDARY ELEMENTS ARE NOT DIMENSIONED, SUCH AS WALL CONFIGURATIONS, INCLUDING EXACT DOOR AND WINDOW LOCATIONS, ALCOVES, SLAB SLOPES AND DEPRESSIONS, CURBS, ETC. VERTICAL DIMENSIONAL CONTROL IS DEFINED BY ARCHITECTURAL WALL SECTIONS AND BUILDING SECTIONS. STRUCTURAL DETAILS SHOW DIMENSIONAL RELATIONSHIPS TO CONTROL DIMENSIONS DEFINED BY ARCHITECTURAL DRAWINGS. DETAILING AND SHOP DRAWING PRODUCTION FOR STRUCTURAL ELEMENTS WILL REQUIRE DIMENSIONAL INFORMATION CONTAINED IN BOTH ARCHITECTURAL AND STRUCTURAL DRAWINGS.

DESIGN CRITERIA

VERTICAL LOADS

AREA	DESIGN DEAD LOAD	LIVE LOAD (2)	PARTITION LOAD	CONCENTRATED LOADS
ROOF	15 PSF	25 PSF (1)		300#
WOOD TRUSSES	15 PSF	25 PSF (1)		
MEZZANINE	15 PSF	60 PSF (3)		

(1) LIVE LOADS EXCEPT SNOW LOADS ARE REDUCED PER IBC SECTION 1607.10. (2) LIVE LOAD REDUCTION NOT PERMITTED EXCEPT AS NOTED IN IBC SECTION 1607.10.

SNOW: (MINIMUM ROOF SNOW LOAD = 25 PSF)

LATERAL FORCES

LATERAL FORCES ARE TRANSMITTED BY DIAPHRAGM ACTION OF ROOF AND FLOORS TO BRACED FRAME/SHEAR WALLS. LOADS ARE THEN TRANSFERRED TO FOUNDATION BY BRACED FRAME/SHEAR WALL ACTION WHERE ULTIMATE DISPLACEMENT IS RESISTED BY PASSIVE PRESSURE OF EARTH AND/OR SLIDING FRICTION. OVERTURNING IS RESISTED BY DEAD LOAD OF THE STRUCTURE.

THE BUILDING MEETS THE CRITERIA TO USE THE "METHOD 2 - SIMPLIFIED ENVELOPE PROCEDURE" PER ASCE 7-10.

- EXPOSURE CATEGORY = B
- BASIC WIND SPEED, (3 SEC. GUST), V_{ULT} = 110 MPH
- RISK CATEGORY PER TABLE 1.5-1 = II
- TOPOGRAPHIC FACTOR $K_{ZT} = 1.0$ - INTERNAL PRESSURE COEFFICIENT (ENCLOSED) = ± 0.18
- COMPONENTS AND CLADDING LOADS, SEE THE FOLLOWING TABLES:

ROOF SURFACES ¹							
	POSITIVE PRESSURES (PSF)			NEGATIVE PRESSURES (PSF)			
EFFECTIVE WIND AREA	ZONE ²						
	1	2	3	1	2	3	
10 SF	19.9	19.9	19.9	-21.8	-25.5	-25.5	
20 SF	19.4	19.4	19.4	-20.7	-24.3	-24.3	
50 SF	18.6	18.6	18.6	-19.2	-22.9	-22.9	
100 SF	18.1	18.1	18.1	-18.1	-21.8	-21.8	

WALL SURFACES AND ROOF OVERHANGS ¹						
	POSITIVE PRE	ESSURE (PSF)	NEGATIVE PRESSURE (PSF)		ROOF OVERHANGS (PSF)	
EFFECTIVE WIND AREA		ZONE ²				
	4	5	4	5	2	3
10 SF	21.8	21.8	-23.6	-29.1	-36.9	-36.9
20 SF	20.8	20.8	-22.6	-27.2	-35.8	-35.8
50 SF	19.5	19.5	-21.3	-24.6	-34.3	-34.3
100 SF	18.5	18.5	-20.4	-22.6	-33.2	-33.2
500 SF	16.2	16.2	-18.1	-18.1	-	-

1. VALUES SHOWN IN TABLE ARE GROSS ULTIMATE WIND PRESSURES.

2. ZONES ARE AS DEFINED BY FIGURE 30.5-1 IN ASCE 7-10.

SEISMIC: (ASCE 7-10) V = CsW
SEISMIC: (ASCE 7-10) V = CsW WHERE $Cs = \frac{S_{DS}}{(\frac{R}{Ie})}$; WITH
Cs MINIMUM = $0.044 \text{ S}_{DS}I_{E} \ge 0.01$ OR Cs MINIMUM = $\frac{0.5S_{1}}{R}$ FOR $S_{1} > 0.0$
$Cs \ MAXIMUM = \frac{\frac{S_{D1}}{T(\frac{R}{Ie})} \ FOR \ T \le T_{L}}{OR}$ $OR \frac{S_{D1}T_{L}}{T^{2} \ (\frac{R}{Ie})} \ FOR \ T > T_{L}$

SEISMIC IMPORTANCE FACTOR, Ie = 1.0 RISK CATEGORY OF BUILDING PER TABLE 1.5-1 = IISPECTRAL RESPONSE ACCELERATIONS Ss = $1.24 \& S_1 = 0.474$ SITE CLASS PER TABLE 20.3-1 = E DESIGN SPECTRAL RESPONSE ACCELERATIONS S_{DS} = 0.744 & S_{D1} = 0.758 SEISMIC DESIGN CATEGORY = D W = EFFECTIVE SEISMIC WEIGHT OF BUILDING = 195 K ANALYSIS PROCEDURE USED = EQUIVALENT LATERAL FORCE PROCEDURE RESPONSE MODIFICATION FACTOR PER TABLE 12.2-1, R = 6.5 Cs = 0.144DESIGN BASE SHEAR V = 28.1 K

PIPES, DUCTS AND MECHANICAL EQUIPMENT SUPPORTED OR BRACED FROM STRUCTURE. CONFORM TO SHEET METAL AND AIR CONDITIONING CONTRACTORS' NATIONAL ASSOCIATION, INC. PUBLICATION "SEISMIC RESTRAINT MANUAL: GUIDELINES FOR MECHANICAL SYSTEMS". SPRINKLER LINE ATTACHMENTS SHALL CONFORM TO NFPA PAMPHLET 13.

FOUNDATION DESIGN CRITERIA (GEOTECHNICAL ENGINEERING SERVICES REPORT, ST, ANDREW CATHOLIC CHURCH & PARISH HALL BY GEO ENGINEERS DATED JUNE 25TH, 2019).

SOIL BEARING PRESSURE: 1500 PSF

PASSIVE RESISTANCE: 200 PCF (INCLUDES F.O.S. ≥ 1.5) COEFFICIENT OF FRICTION: 0.4 (INCLUDES F.O.S. ≥ 1.5) *1/3 INCREASE ALLOWED FOR SEISMIC OR WIND LOADING

ALL FOOTINGS SHALL BEAR ON FIRM, UNDISTURBED EARTH OR "STRUCTURAL BACKFILL". NATIVE EARTH BEARING SHALL BE SURFACE COMPACTED. AREAS OVER-EXCAVATED SHALL BE BACKFILLED WITH LEAN CONCRETE (F'c= 2000 PSI) OR "STRUCTURAL BACKFILL". AREAS DESIGNATED "STRUCTURAL BACKFILL" SHALL BE FILLED WITH APPROVED WELL-GRADED BANKRUN MATERIAL. MAXIMUM SIZE OF ROCK 6". FROZEN SOIL, ORGANIC MATERIAL AND DELETERIOUS MATTER NOT ALLOWED. COMPACT TO AT LEAST 95% OF ITS MAXIMUM DENSITY AS DETERMINED BY ASTM D1557. CONTRACTOR SHALL EXERCISE EXTREME CARE DURING EXCAVATION TO AVOID DAMAGE TO BURIED LINES, TANKS, AND OTHER CONCEALED ITEMS. UPON DISCOVERY, DO NOT PROCEED WITH WORK UNTIL RECEIVING WRITTEN INSTRUCTIONS FROM ARCHITECT. A COMPETENT REPRESENTATIVE OF THE OWNER SHALL INSPECT ALL FOOTING EXCAVATIONS FOR SUITABILITY OF BEARING SURFACES PRIOR TO PLACEMENT OF REINFORCING STEEL. PROVIDE DRAINAGE AND DEWATERING AROUND ALL WORK TO AVOID WATER-SOFTENED FOOTINGS.

CONCRETE

MIX DESIGNS: THE CONTRACTOR SHALL DESIGN CONCRETE MIXES THAT MEET OR EXCEED THE REQUIREMENTS OF THE CONCRETE MIX TABLE. THE MIX DESIGNS SHALL FACILITATE ANTICIPATED PLACEMENT METHODS, WEATHER, REBAR CONGESTION, ARCHITECTURAL FINISHES, CONSTRUCTION SEQUENCING, STRUCTURAL DETAILS, AND ALL OTHER FACTORS REQUIRED TO PROVIDE A STRUCTURALLY SOUND, AESTHETICALLY ACCEPTABLE FINISHED PRODUCT. WATER REDUCING ADMIXTURES WILL LIKELY BE REQUIRED TO MEET THESE REQUIREMENTS. CONCRETE MIX DESIGNS SHALL CLEARLY INDICATE THE TARGET SLUMP. SLUMP TOLERANCE SHALL BE ± 1-1/2 INCHES.

AGGREGATE: COARSE AND FINE AGGREGATE SHALL CONFORM TO ASTM C33

CEMENT: CEMENT SHALL CONFORM TO ASTM C150, TYPE II PORTLAND CEMENT, UNLESS NOTED OTHERWISE.

FLYASH: SHALL CONFORM TO ASTM C618 CLASS C OR F, MAXIMUM LOSS OF IGNITION SHALL BE 1.0%.

ALTERNATE MIX DESIGNS: VARIATIONS TO THE MIX DESIGN PROPORTIONS MAY BE ACCEPTED IF SUBSTANTIATED IN ACCORDANCE WITH ACI 318, CHAPTER 19. PROVIDE SUBMITTALS A MINIMUM OF TWO WEEKS PRIOR TO BID FOR DETERMINATION OF ACCEPTABILITY.

ADMIXTURES: ADMIXTURES SHALL BE BY MASTER BUILDERS, W.R. GRACE, OR PRE-APPROVED EQUAL. ALL MANUFACTURER'S RECOMMENDATIONS SHALL BE FOLLOWED.

WATER: SHALL BE CLEAN AND POTABLE.

MAXIMUM CHLORIDE CONTENT: THE MAXIMUM WATER SOLUBLE CHLORIDE CONTENT SHALL NOT EXCEED 0.15% BY WEIGHT OF CEMENTITIOUS MATERIAL UNLESS NOTED OTHERWISE.

CONCRETE EXPOSED TO WEATHER: PROVIDE 5.0% TOTAL AIR CONTENT FOR ALL CONCRETE EXPOSED TO WEATHER. TOTAL AIR CONTENT IS THE SUM OF ENTRAINED AIR PROVIDED BY ADMIXTURES AND NATURALLY OCCURRING ENTRAPPED AIR. AIR CONTENT SHALL BE TESTED PRIOR TO BEING PLACED IN THE PUMP HOPPER OR BUCKET; IT IS NOT REQUIRED TO BE TESTED AT THE DISCHARGE END OF THE PUMP HOSE. THE TOLERANCE ON ENTRAPPED AIR SHALL BE +2.0% AND -1.5% WITH THE AVERAGE OF ALL TESTS NOT LESS THAN THE SPECIFIED AMOUNT.

TOTAL CEMENTITIOUS MATERIAL: THE SUM OF ALL CEMENT PLUS FLYASH AND SLAG. AT THE CONTRACTORS OPTION FLYASH OR SLAG MAY BE SUBSTITUTED FOR CEMENT BUT SHALL NOT EXCEED 25% BY WEIGHT OF TOTAL CEMENTITIOUS MATERIAL. IN NO CASE SHALL THE AMOUNT OF FLYASH OR SLAG BE LESS THAN REQUIRED BY THE CONCRETE MIX DESIGN TABLE. FOOTING MIXES SHALL CONTAIN NOT LESS THAN 5 SACKS OF CEMENTITIOUS MATERIAL PER CUBIC YARD, ALL OTHER MIXES SHALL CONTAIN NOT LESS THAN 5-1/2 SACKS OF CEMENTITIOUS MATERIAL PER CUBIC YARD, UNLESS NOTED OTHERWISE.

ITEM	DESIGN f'c (PSI) (AT 28 DAYS U.N.O.)	MAX. W/C RATIO	MIN. FLYASH OR SLAG (PCY)	AGGREGATE GRADING ASTM AASHTO	NOTES
SLABS ON GRADE - UNO	4000	0.45	100	57 OR 67	1
MAT FOUNDATIONS	5000 @ 56 DAYS	0.50	100	57 OR 67	
STEM WALLS AND OTHER WALLS - UNO	4000	0.50	100	57 OR 67	
ALL OTHER CONCRETE	4000	0.50		57 OR 67	

CONCRETE MIX NOTES

- MAXIMUM WATER CONTENT 240 PCY.
- 2. THIS MIX SHALL CONTAIN 1 GALLON PER CY OF 'ECLIPSE' SHRINKAGE REDUCING ADD MIXTURE BY W.R. GRACE OR APPROVED ALTERNATE. FOR CONCRETE REQUIRING AN AIR ENTRAINMENT ADMIXTURE, 'ECLIPSE PLUS' SHALL BE USED.

CONCRETE PLACEMENT

PLACE CONCRETE FOLLOWING ALL APPLICABLE ACI RECOMMENDATIONS. CONCRETE SHALL BE PROPERLY CONSOLIDATED PER ACI 309 USING INTERIOR MECHANICAL VIBRATORS, DO NOT OVER-VIBRATE. CONCRETE SHALL BE POURED MONOLITHICALLY BETWEEN CONSTRUCTION OR EXPANSION JOINTS. IF CONCRETE IS PLACED BY THE PUMP METHOD, HORSES SHALL BE PROVIDED TO SUPPORT THE HOSE, THE HOSE SHALL NOT BE ALLOWED TO RIDE ON THE REINFORCING. WEATHER FORECASTS SHALL BE MONITORED AND ACI RECOMMENDATIONS FOR HOT AND COLD WEATHER CONCRETING SHALL BE FOLLOWED AS REQUIRED. CONCRETE SHALL NOT FREE FALL MORE THAN 5 FEET DURING PLACEMENT WITHOUT WRITTEN APPROVAL OF STRUCTURAL ENGINEER.

FLOATING & FINISHING OPERATIONS

WATER SHALL NOT BE ADDED TO THE CONCRETE SURFACE DURING FLOATING & FINISHING OPERATIONS. PRE-APPROVED EVAPORATION RETARDER SPECIFICALLY DESIGNED FOR FLOATING & FINISHING OPERATIONS ARE ACCEPTABLE.

COLD WEATHER PLACEMENT:

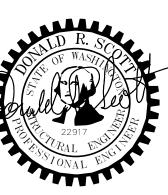
- 1. COLD WEATHER IS DEFINED BY ACI 306 AS "A PERIOD WHEN FOR MORE THAN 3 SUCCESSIVE DAYS THE MEAN DAILY TEMPERATURE DROPS BELOW 40° F."
- 2. NO CONCRETE SHALL BE PLACED ON FROZEN OR PARTIALLY FROZEN GROUND. THAWING THE GROUND WITH HEATERS IS PERMISSIBLE.
- CONCRETE MIX TEMPERATURES SHALL BE AS SHOWN BELOW. HEATING OF WATER AND/OR AGGREGATES MAY BE REQUIRED TO ATTAIN THESE TEMPERATURES.
- 4. THE CONCRETE MAY REQUIRE PROTECTION FOR 4-7 DAYS AFTER POURING. IF TEMPERATURES REMAIN BELOW FREEZING, INSULATING BLANKET COVERAGE IS REQUIRED. IF TEMPERATURES ARE SLIGHTLY BELOW FREEZING (30° F MIN.) AT NIGHT AND ABOVE FREEZING DURING THE DAY, KRAFT PAPER WITH COMPLETE COVERAGE MAY BE USED IN LIEU OF INSULATED BLANKETS.
- 5. NO ADDITIVES CONTAINING CHLORIDES SHALL BE USED. USE "POZZUTEC 20+" BY MASTER BUILDERS OR "POLARSET" BY W.R. GRACE OR PRE-APPROVED EQUAL

CONDITION OF PLACEMENT AND CURING	WALLS & SLABS	FOOTINGS	
MIN. TEMP. FRESH CONCRETE AS MIXED FOR WEATHER INDICATED, DEGREES F.	ABOVE 30° F. 0° TO 30° F. BELOW 0° F.	60° 65° 70°	55° 60° 65°
MIN. TEMP. FRESH CONCRETE AS PLACED AN	55°	50°	
MAX. ALLOWABLE GRADUAL DROP IN TEMP. THE HOURS AFTER END OF PROTECTION, DEGREE	50°	40°	

SHEET NUMBER SHEET DESCRIPTION				
5001	GENERAL NOTES			
6002	GENERAL NOTES			
8003	GENERAL NOTES			
6004	GENERAL NOTES			
S101	FOUNDATION PLAN			
3102	ROOF FRAMING PLAN			
3201	FOUNDATION DETAILS			
301	FRAMING DETAILS			
3302	FRAMING DETAILS			
3303	FRAMING DETAILS			
304	FRAMING DETAILS			







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1851 Date: 2/7/2020

GENERAL NOTES

Drawn By:

Checked By:

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HOT OR WINDY WEATHER PLACEMENT

HOT WEATHER IS DEFINED BY ACI 305 AS "ANY COMBINATION OF HIGH AIR TEMPERATURE, LOW RELATIVE HUMIDITY, AND WIND VELOCITY, TENDING TO IMPAIR THE QUALITY OF FRESH HARDENED CONCRETE. ACI 305 FIGURE 2.1.5 SHALL BE USED BY THE CONTRACTOR TO ESTIMATE THE RATE OF EVAPORATION. WHEN THE ESTIMATED RATE OF EVAPORATION IS GREATER THAN 0.2 PSF/HOUR THE PLACEMENT SHALL BE CONSIDERED A HOT WEATHER PLACEMENT. PRECAUTIONS AGAINST PLASTIC SHRINKAGE CRACKING ARE NECESSARY. PRECAUTIONS TAKEN BY THE CONTRACTOR VARY DEPENDING UPON THE FACTORS ASSOCIATED WITH WATER EVAPORATION AND INCLUDE BUT ARE NOT LIMITED TO:

1. LIMITING CONCRETE TEMPERATURE TO 100°F AT TIME OF PLACEMENT.

2. APPLICATION OF AN EVAPORATION RETARDER.

3. USE OF FOG SPRAY.

4. REDUCTION OF POUR SIZE.

5. PLACING CONCRETE AT NIGHT.

CONTROL AND CONSTRUCTION JOINTS

CONSTRUCTION JOINTS SHALL MEET THE REQUIREMENTS OF ACI 301 SECTIONS 2.2.2.5 AND 5.3.2.6. SPECIAL BONDING METHODS PER SECTION 5.3.2.6 SHALL BE SATISFIED BY ITEM 4 BELOW UNLESS OTHERWISE DETAILED ON THE STRUCTURAL DRAWINGS. WHERE CONSTRUCTION JOINTS ARE NOT SHOWN ON PLAN OR ADDITIONAL CONSTRUCTION JOINTS ARE REQUIRED SUBMIT PROPOSED JOINTING FOR STRUCTURAL ENGINEERS APPROVAL. PROVIDE CONSTRUCTION JOINTS AS INDICATED BELOW UNLESS NOTED OTHERWISE ON THE PLANS:

1. SLABS ON GRADE: PROVIDE CONSTRUCTION AND/OR CONTROL JOINTS AT 16 FEET O.C. MAXIMUM FOR UNEXPOSED SLABS ON GRADE AND 12 FEET O.C. FOR EXPOSED SLABS ON GRADE. COORDINATE JOINTS WITH ARCHITECTURAL DRAWINGS.

EMBEDDED ITEMS

1. NO ALUMINUM ITEMS SHALL BE EMBEDDED IN ANY CONCRETE.

CONCRETE CURING AND SEALING

CURING PROCEDURES SHALL COMMENCE IMMEDIATELY AFTER FINISHING CONCRETE TO MAINTAIN CONCRETE IN A MOIST CONDITION. VERIFY CURING AND/OR SEALING PRODUCTS ARE COMPATIBLE WITH FLOOR COVERINGS SHOWN ON THE ARCHITECTURAL DRAWINGS. FOLLOW ALL MANUFACTURER'S RECOMMENDATIONS. SLABS ARE DEFINED AS SLABS ON GRADE, CONCRETE ON METAL DECK, ELEVATED POST-TENSIONED OR MILD REINFORCED DECKS, AND TOPPING SLABS.

ITEM	CONCRETE CURING NOTES
SLABS	1, (2 OR 3 OR 4)
ALL OTHER CONCRETE	NONE

CONCRETE CURING NOTES:

- 1. WHEN THE ESTIMATED EVAPORATION RATE IS GREATER THAN 0.2 PSF/HOUR PROVIDE A SPRAY APPLIED EVAPORATION RETARDER IMMEDIATELY AFTER CONCRETE PLACEMENT. THE EVAPORATION RATE MAY BE CALCULATED PER ACI 305 FIGURE 2.1.5.
- 2. PROVIDE PRE-APPROVED CONTINUOUS WET CURE METHOD FOR A MINIMUM OF 14 DAYS.
- 3. APPLY A LIQUID MEMBRANE FORMING CURING COMPOUND, CONFORMING TO ASTM C309 TYPE 1 CLASS B SPECIFICATIONS OR ASTM C1315 TYPE 1 CLASS A SPECIFICATIONS, PER MANUFACTURER'S RECOMMENDATIONS IMMEDIATELY AFTER FINAL FINISHING. CURING COMPOUND SHALL BE COMPATIBLE WITH ARCHITECTURAL FLOOR COVERINGS AND SEALERS.
- 4. PROVIDE 'ULTRACURE MAX' MOISTURE RETAINING COVER BY MCTECH GROUP, OR APPROVED EQUAL, FOR A MINIMUM OF 14 DAYS.

<u>GROUT</u>

NON-SHRINK GROUT: MASTER BUILDERS "MASTERFLOW 928" OR PRE-APPROVED EQUAL. GROUT SHALL CONFORM TO CRD-C621 AND ASTM C1107 WHEN TESTED AT A FLUID CONSISTENCY PER CRD-C611-85 FOR 30 MINUTES. GROUT MAY BE PLACED FROM A 25 SECOND FLOW TO A STIFF PACKING CONSISTENCY. FILL OR PACK ENTIRE SPACE UNDER PLATES OR SHAPES. FOLLOW MANUFACTURER'S RECOMMENDATIONS FOR PREPARATION, INSTALLATION, AND CURING.

REINFORCING STEEL

REINFORCING STEEL SHALL CONFORM TO:

ASTM A615, GRADE 60 TYPICAL UNLESS NOTED OTHERWISE.

DETAIL FABRICATE AND PLACE PER ACI 315 AND ACI 318.

REINFORCING SPLICE AND DEVELOPMENT LENGTH SCHEDULE, Fy=60 KSI (UNLESS NOTED OTHERWISE)								
BAR	MINIMUM LAP SPLI	CE LENGTHS ("Ls")	MINIMUM DEVELOPI	MENT LENGTHS ("Ld")	MINIMUM EMBEDMENT LENGTH FOR			
SIZE	TOP BARS (1)	OTHER BARS	TOP BARS (1)	OTHER BARS	STANDARD END HOOKS ("Ldh")			
#3	2'-0"	1'-6"	1'-6"	1'-3"	0'-7"			
#4	2'-8"	2'-0"	2'-0"	1'-7"	0'-9"			
#5	3'-4"	2'-7"	2'-7"	2'-0"	1'-0"			
#6	4'-0"	3'-1"	3'-1"	2'-4"	1'-2"			
#7	5'-10"	4'-6"	4'-6"	3'-6"	1'-5"			
#8	6'-8"	5'-2"	5'-2"	3'-11"	1'-7"			

SPLICE TABLE NOTES:

1. "TOP BARS" ARE HORIZONTAL BARS WITH MORE THAN 12" DEPTH OF CONCRETE CAST BELOW THEM.

REINFORCING STEEL COVER

PROVIDE CONCRETE COVER OVER REINFORCEMENT AS FOLLOWS, UNLESS NOTED OTHERWISE:

CONCRETE CAST AGAINST EARTH ------ 3"
EXPOSED TO WEATHER OR EARTH ------ 2"
TIES ON BEAMS AND COLUMNS ----- 1-1/2"
WALLS AND SLABS NOT EXPOSED TO WEATHER---- 3/4"

POST-INSTALLED ANCHORS

POST-INSTALLED ANCHORS: SHALL ONLY BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS. THE CONTRACTOR SHALL OBTAIN APPROVAL FROM THE STRUCTURAL ENGINEER PRIOR TO INSTALLING POST-INSTALLED ANCHORS IN PLACE OF MISSING OR MISPLACED CAST-IN-PLACE ANCHORS. CARE SHALL BE TAKEN IN PLACING POST-INSTALLED ANCHORS TO AVOID CONFLICTS WITH REBAR. INSTALL IN ACCORDANCE WITH THE MANUFACTURER'S PUBLISHED INSTALLATION INSTRUCTIONS. INSTALLER SHALL BE QUALIFIED AND TRAINED BY THE MANUFACTURER. HOLES SHALL BE HAMMER DRILLED ONLY (ROTARY DRILLED ONLY AT UNREINFORCED MASONRY - NO HAMMER TOOLS).

CONCRETE ANCHORS:

- ADHESIVE ANCHORS: HILTI HIT-HY 200 (ICC-ESR-3187)
 - *CONCRETE SHALL BE A MINIMUM OF 21 DAYS OLD AT TIME OF INSTALLATION.
 - *CONCRETE SHALL BE IN THE TEMPERATURE RANGE AS REQUIRED BY THE CONCRETE
 - MANUFACTURER.
 - *HOLE SHALL BY HAMMER-DRILLED ONLY.
 - *HOLE SHALL BE DRY AT TIME OF INSTALLATION.
 - *INSTALLER OF HORIZONTAL OR UPWARDLY INCLINED (ANY POSITION EXCEPT DIRECTLY DOWNWARD) ANCHORS SHALL ALSO BE CERTIFIED BY THE ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM.
- EXPANSION ANCHORS: KWIKBOLT TZ (ICC ESR-1917) BY HILTI, INC. OR STRONG-BOLT 2 (ICC ESR-3037) BY
- SIMPSON STRONG TIE, INC.
 SCREW ANCHORS: KWIK HUS-EZ (ICC ESR-3027) BY HILTI, INC. OR TITEN HD (ICC ESR-2713) BY SIMPSON STRONG TIE, INC.

WELDING

STRUCTURAL STEEL: WELD IN ACCORDANCE WITH "STRUCTURAL WELDING CODE" AWS D1.1.

<u>CERTIFICATION</u>: ALL WELDING SHALL BE PERFORMED BY WABO/AWS CERTIFIED WELDERS. WELDERS SHALL BE PREQUALIFIED FOR EACH POSITION AND WELD TYPE WHICH THE WELDER WILL BE PERFORMING.

CARPENTRY:

NAILS: CONNECTION DESIGNS ARE BASED ON "COMMON WIRE" NAILS WITH THE FOLLOWING PROPERTIES:

PENNYWEIGHT	DIAMETER (INCHES)	LENGTH (INCHES)	TRACKER** EMBOSSED HEAD / COLOR
8d	0.131	2-1/2	3 / BLUE
10d	0.148	3	4 / WHITE
16d	0.162	3-1/2	6 / ORANGE
20d	0.192	4	-

FOR DIAPHRAGM OR SHEAR WALL NAILING THE FOLLOWING FASTENER TYPES MAY BE USED AT EQUIVALENT SPACING TO THAT SPECIFIED ON PLANS:

FASTENER TYPE	DIAMETER (INCHES)	LENGTH (INCHES)	EQUIV	EQUIVALENT SPACING (INCHES)		TRACKER** EMBOSSED HEAD / COLOR
8d COMMON WIRE	0.131	2-1/2	6	4	3	3 / BLUE
8d "DIPPED GALV. BOX" 8d "SHINY BOX" 12 GA. STAPLES 14 GA. STAPLES 15 GA STAPLES	0.131 0.113 0.1055 0.080 0.072	2-1/2 2-1/2 1-7/8* 1-1/2* 1-1/2*	6 4-1/2 6 6 5	4 3 5-1/2 4 3	3 2-1/2 4 3 2-1/2	E3 / NONE 1 / BLUE - -
10d COMMON WIRE	0.148	3	6	4	3	4 / WHITE
10d "HOT DIPPED GALV. BOX" 10d "SHINY BOX"	0.148 0.128	3 3	6 4-1/2	4 3	3 2-1/4	F4 / NONE 3 / WHITE

*BASED ON 15/32" PLYWOOD OR OSB.

**REFERENCE TO EMBOSSED HEAD / COLOR CODED NAILS PER TRACKERS SYSTEM.

WOOD SHEATHING (STRUCTURAL): SHEATHING ON ROOF SURFACES SHALL BE PLYWOOD ONLY. SHEATHING ON FLOOR AND WALLS SHALL BE PLYWOOD OR ORIENTED STRAND BOARD (OSB). PLYWOOD SHEATHING SHALL BE 5-PLY MINIMUM WHERE INDICATED AS PERFORMANCE CATAGORY 3/4" OR THICKER. WOOD SHEATHING SHALL BE "STRUCTURAL I" CONFORMING TO PS1-09 AND/OR PS2-10. ALL PANELS SHALL BEAR THE STAMP OF AN APPROVED GRADING AGENCY. SPAN RATING SHALL BE PROVIDED AS FOLLOWS: ROOF FRAMING AT 32"O.C. (48/24); ROOF FRAMING AT 24"O.C. (32/16); WALLS (32/16); FLOORS (48/24) ALL WOOD SHEATHED WALLS SHALL BE BLOCKED AT ALL PANEL EDGES UNLESS NOTED OTHERWISE.

GLUE-LAMINATED MEMBERS: CONFORM TO ANSI/AITC A190.1. MEMBERS SHALL BE COMBINATION 24F-V4 DOUGLAS FIR (DF) FOR SIMPLE SPANS AND 24F-V8 DF FOR CANTILEVERED SPANS (Fb=2400 PSI, Fv=265 PSI, E= 1.8X10^6 PSI) AND DF COMBINATION 2 FOR COLUMNS.





Archdiocese of Seattle

St. Andrew Catholic Church Paris

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 PERMIT

GENERAL NOTES



S002

<u>SPECIES AND GRADE</u> (BASE DESIGN VALUE)

- 6x BEAMS AND HEADERS. "DOUG FIR-LARCH" NO. 1 (Fb=1350 PSI, Fv=170 PSI)
- 2x TO 4x JOISTS, PURLINS AND HEADERS. "DOUG FIR-LARCH" NO. 2 (Fb=900 PSI, Fv=180 PSI) OR "HEM-FIR" NO. 1 (Fb=975 PSI, Fv=150 PSI)
- 6x POSTS AND COLUMNS. "DOUG FIR-LARCH" NO. 1 (Fc=1000 PSI)
- EXTERIOR STUDS, INTERIOR BEARING WALLS AND 4x COLUMNS. "DOUG FIR-LARCH" NO. 2 (Fb= 900 PSI, Fc=1350 PSI) OR "HEM-FIR" NO. 1 (Fb=975 PSI, Fc=1350 PSI).
- INTERIOR NON-BEARING STUD WALLS. "DOUG FIR-LARCH" NO. 2 (Fb=900 PSI. Fc=1350 PSI) OR "HEM-FIR" NO. 1 (Fb=975 PSI, Fc=1350 PSI)
- 2x & 3x T&G DECKING: "DOUG FIR-LARCH" COMMERCIAL (Fb=1450 PSI, E=1700 KSI)
- THE MINIMUM GRADE OF ALL OTHER STRUCTURAL FRAMING. "DOUG FIR-LARCH" NO. 2 (Fb= 900 PSI,
- Fc=1350 PSI), OR "HEM-FIR" NO. 1 (Fb=975 PSI, Fc=1350 PSI). 8. UTILITY & STANDARD GRADES NOT PERMITTED.

STRUCTURAL COMPOSITE LUMBER (SCL): SHALL BE MANUFACTURED BY REDBUILT LLC., OR PRE-APPROVED EQUAL IN ACCORDANCE WITH APPROVED SHOP AND INSTALLATION DRAWINGS CONFORMING TO A CURRENT **EVALUATION REPORT.**

MIINIMUM DESIGN VALUES:

- 2x SCL: Fb = 1700 PSI, Fv = 285 PSI, E = 1300 KSI
- 1-3/4" SCL: Fb = 2600 PSI, Fv = 285 PSI, E = 1800 KSI
- 3-1/2" SCL: Fb = 2900 PSI, Fv = 285 PSI, E = 2000 KSI 5-1/4" SCL: Fb = 2900 PSI, Fv = 285 PSI, E = 2000 KSI
- RIMBOARD: APA/EWS PERFORMANCE RATED RIM (PRR-401) 1-1/4" MINIMUM THICKNESS

MEMBERS HAVE BEEN DESIGNED TO SERVICEABILITY AND OTHER PERFORMANCE BASED REQUIREMENTS, WHICH MAY EXCEED MINIMUM DESIGN LOADS AND CODE REQUIREMENTS. SUBSTITUTIONS MUST MEET OR EXCEED MOMENT, SHEAR, AND STIFFNESS OF THOSE MEMBERS SPECIFIED AT THE SAME DEPTH AND SPACING.

PRESERVATIVE TREATED WOOD REQUIREMENTS:

TREATMENTS OTHER THAN THOSE LISTED BELOW ARE NOT PERMITTED.

		APPLICATION	SPECIFIED MATERIAL	PRESERVATIVE TREATMENT (1)	CONNECTORS & FASTENERS (2)(3)
	Ϋ́	FOUNDATION SILL PLATES, TOP PLATES & LEDGERS ON	2x, 4x, 6x (FIR), OR GLULAM (SP)	SBX	GALV (G60)
SURE	DRY	CONCRETE OR MASONRY WALLS (4)		ACQ, CBA, CA	GALV (G185)
OSI		FRAMING, DECKING,	2x, & 4x (FIR)	ACQ, CBA, CA	GALV (G185)
EXPO	ET	POSTS & LEDGERS	2x, & 4x (CEDAR)	NONE	GALV (G90)
	WE	BEAMS & COLUMNS	6x (FIR), OR GLULAM (SP)	ACQ, CBA, CA	GALV (G185)
			6x OR GLULAM (CEDAR)	NONE	GALV (G90)

FIR: DOUG-FIR OR HEM-FIR

SP: SOUTHERN PINE

- CCA: CHROMATED COPPER ARSENATE NOT PERMITTED SBX: DOT SODIUM BORATE
 - ACQ: ALKALINE COPPER QUAT
- CBA & CA: COPPER AZOLE
- CONNECTORS: JOIST HANGERS, STRAPS, FRAMING CONNECTORS, COLUMN CAPS AND BASES, ETC. FASTENERS: MACHINE BOLTS, ANCHOR BOLTS AND LAG SCREWS WITH ASSOCIATED PLATE WASHERS AND NUTS. NAILS, SPIKES, WOOD SCREWS, ETC.
- G60, G90 & G185 PER ASTM A653 FOR COLD-FORMED STEEL CONNECTORS. BATCH/POST HOT-DIP GALVANIZED PER ASTM A123 FOR CONNECTORS AND ASTM A153 STRUCTURAL STEEL CONNECTORS. HOT-DIP GALVANIZED PER ASTM A153 FOR FASTENERS OR MECHANICALLY GALVANIZED FASTENERS PER ASTM B695, CLASS 55 OR GREATER.
- AT CONTRACTORS OPTION, LEDGERS AND TOP PLATES A MINIMUM OF 8 FEET ABOVE GRADE ON CONCRETE OR MASONRY WALLS MAY BE UN-TREATED IF COMPLETELY SEPARATED FROM THE WALL BY A SELF ADHERING ICE & WATER SHIELD BARRIER (40 MIL MINIMUM).

GENERAL REQUIREMENTS: PROVIDE MINIMUM NAILING PER IBC TABLE 2304.10.1 OR MORE, AS OTHERWISE SHOWN. STAGGER ALL NAILING TO PREVENT SPLITTING OF WOOD MEMBERS. ALL WOOD IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESERVATIVE TREATED WITH THE EXCEPTION OF INTERIOR CONCRETE TOPPINGS ON WOOD FLOOR SYSTEMS. HOLES AND CUTS IN 3x OR 4x PLATES SHOULD BE TREATED WITH A 9% SOLUTION OF COPPER NAPHTHENATE. BOLT HOLES IN WOOD MEMBERS SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN THE BOLT DIAMETER. PROVIDE CUT WASHERS WHERE BOLT HEADS, NUTS AND LAG SCREW HEADS BEAR ON WOOD. PROVIDE A MINIMUM 3"x3"x0.229" PLATE WASHER ON ALL ANCHOR BOLTS WHICH CONNECT MUD SILLS TO FOUNDATION. DO NOT NOTCH OR DRILL STRUCTURAL MEMBERS, EXCEPT AS ALLOWED BY IBC SECTIONS 2308.4.2.4, 2308.5.9, 2308.5.10 AND 2308.7.4 OR AS RESTRICTED BY PLANS OR DETAILS OR AS APPROVED PRIOR TO INSTALLATION. REFER TO PRESERVATIVE TREATED WOOD REQUIREMENTS IN THESE GENERAL NOTES FOR GALVANIZING REQUIREMENTS FOR CONNECTORS AND FASTENERS.

WOOD SHRINKAGE AND CONSOLIDATION: SHRINKAGE OF WOOD MEMBERS AND CONSOLIDATION OF BEARING WALLS IS EXPECTED FROM TIME OF FRAMING UNTIL AFTER BUILDING IS PUT IN SERVICE. MECHANICAL, ELECTRICAL, AND PLUMBING SYSTEMS SHALL BE CONSTRUCTED TO ACCOMODATE 1/4" OF TOTAL SETTLEMENT PER STORY.

FRAMING CONNECTORS: SHALL CONFORM TO CURRENT EVALUATION REPORT AND BE MANUFACTURED BY SIMPSON STRONG-TIE COMPANY, SAN LEANDRO, CA., OR PRE-APPROVED EQUAL. PROVIDE MAXIMUM SIZE AND QUANTITY OF NAILS OR BOLTS PER MANUFACTURER, EXCEPT AS NOTED OTHERWISE. PROVIDE LEAD HOLES AS REQUIRED TO PREVENT SPLITTING OF WOOD MEMBERS. REFER TO PRESERVATIVE TREATED WOOD REQUIREMENTS IN THESE GENERAL NOTES FOR GALVANIZING REQUIREMENTS FOR CONNECTORS AND FASTENERS.

METAL-PLATE-CONNECTED WOOD TRUSSES: SHALL BE MANUFACTURED BY AN APPROVED TRUSS MANUFACTURER IN ACCORDANCE WITH APPROVED SHOP AND INSTALLATION DRAWINGS. TRUSS CALCULATION PACKAGE SHALL BE DESIGNED UNDER THE DIRECT SUPERVISION OF A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF PROJECT PER IBC SECTION 2303.4 TO CARRY THE LOADS LISTED IN THE DESIGN CRITERION AND ANY ADDITIONAL LOADS INDICATED ON THE FRAMING PLANS AND DETAILS. THE TRUSS ENGINEER SHALL ASSUME ALL RESPONSIBILITY FOR THE WORK OF ALL SUBORDINATES INVOLVED IN THE PREPARATION OF THE TRUSS PLACEMENT PLANS AND TRUSS DESIGN DRAWINGS. ALL ROOF TRUSSES ARE TO BE PRE-ENGINEERED. ROOF TRUSSES SHALL BE PROVIDED TO COMPLETE THE ROOF FRAMING FROM THE ROOF SHEATHING TO THE SUPPORTING MEMBERS BELOW. TRUSSES DESIGNATED ON PLANS ARE FOR TYPICAL UNIFORMLY LOADED CONDITIONS. TRUSS ENGINEER SHALL PROVIDE ADDITIONAL TRUSSES AS REQUIRED TO SUPPORT SPECIAL LOADING CONDITIONS INDICATED ON DRAWINGS. PROVIDE SHOP AND INSTALLATION DRAWINGS AND CALCULATIONS PRODUCED UNDER THE SUPERVISION OF AND BEARING THE STAMP OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF PROJECT. DETAIL DRAWINGS TO INDICATE ALL INFORMATION AS REQUIRED IN IBC SECTION 2303.4.1. ALONG WITH THE FOLLOWING:

*KEY PLAN SHOWING EACH TRUSS

*INDIVIDUAL TRUSS DESIGNS

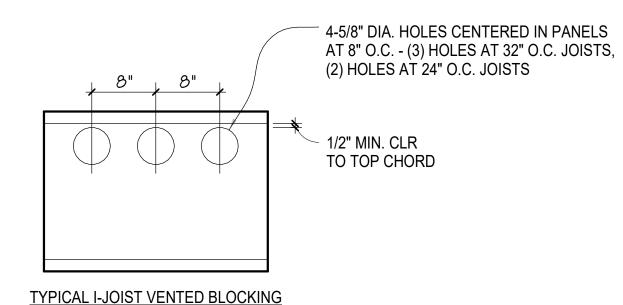
*PERMANENT BRACING REQUIREMENTS INCLUDING PLACEMENT AND CONNECTIONS DETAILS *TRUSS DRAWINGS SHALL SPECIFY ALL TRUSS CONNECTIONS/HARDWARE TO MEET THE REQUIREMENTS OF THE PLAN.

TRUSS DESIGN CALCULATIONS SHALL BE PROVIDED FOR STANDARD LOADING ALONG WITH DESIGN CHECKS FOR SPECIAL LOADING CONDITIONS WHICH INCLUDE FREE BODY DIAGRAMS, LOADING BREAK DOWN, DESCRIPTION OF LOADS (I.E. MECH UNIT, SUSPENDED WALL, ETC.) AND THE RATIONALE FOR LOADING DISTRIBUTION ON MULTIPLE MEMBERS. SUBMITTAL SHALL ALSO PROVIDE ANY DOCUMENTATION NECESSARY TO INTERPRET DATA INDICATED ON CALCULATIONS.

REFER TO THE <u>FRAMING CONNECTORS</u> SECTION OF THESE GENERAL NOTES FOR REQUIREMENTS PLACED UPON CONNECTOR HARDWARE SPECIFIED BY TRUSS ENGINEER AND/OR PROVIDED BY TRUSS MANUFACTURER.

PROVIDE CERTIFICATE OF CONFORMANCE FROM AN INDEPENDENT TESTING LABORATORY OR A LICENSED PROFESSIONAL ENGINEER CERTIFYING THAT THEY HAVE INSPECTED THE FINISHED TRUSSES AND THAT ALL TRUSSES ARE CONSTRUCTED IN CONFORMANCE WITH THE TRUSS DESIGN DRAWINGS.

OPEN-WEB TRUSSES AND I-JOISTS: SHALL BE MANUFACTURED BY REDBUILT LLC, OR PRE-APPROVED EQUAL IN ACCORDANCE WITH APPROVED SHOP AND INSTALLATION DRAWINGS. MEMBERS SHALL BE DESIGNED UNDER THE DIRECT SUPERVISION OF A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF PROJECT. THE ENTIRE OPEN-WEB TRUSS/I-JOIST ASSEMBLY SHALL BE AS APPROVED BY CURRENT EVALUATION REPORT. MEMBERS SHALL BE DESIGNED TO CARRY THE LOADS LISTED IN THE DESIGN CRITERION AND ANY ADDITIONAL LOADS INDICATED ON THE FRAMING PLANS AND DETAILS. THE TRUSS ENGINEER SHALL ASSUME ALL RESPONSIBILITY FOR THE WORK OF ALL SUBORDINATES INVOLVED IN THE PREPARATION OF THE TRUSS PLACEMENT PLANS AND TRUSS DESIGN DRAWINGS. TRUSSES/I-JOISTS SHALL BE PROVIDED TO COMPLETE THE ROOF AND/OR FLOOR FRAMING FROM THE SHEATHING TO THE SUPPORTING MEMBERS BELOW. MEMBER DESIGNATIONS ON PLANS ARE FOR TYPICAL UNIFORMLY LOADED CONDITIONS. MANUFACTURER SHALL PROVIDE ADDITIONAL MEMBERS AS REQUIRED TO SUPPORT SPECIAL LOADING CONDITIONS INDICATED ON DRAWINGS. PROVIDE SHOP AND INSTALLATION DRAWINGS AND CALCULATIONS PRODUCED UNDER THE SUPERVISION OF AND BEARING THE STAMP OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF PROJECT. DETAIL DRAWINGS TO INDICATE MEMBER TYPES, SIZE, SPACING, BRIDGING, BLOCKING, CONNECTIONS, ANCHORING, BEARING PLATE AND OTHER PERTINENT DETAILS. PROVIDE 1 1/2" DIA. OPEN KNOCKOUTS AT 12" O.C. ON ALL ROOF I-JOISTS.



MEMBER DESIGN CALCULATIONS SHALL BE PROVIDED FOR STANDARD LOADING ALONG WITH DESIGN CHECKS FOR SPECIAL LOADING CONDITIONS WHICH INCLUDE FREE BODY DIAGRAMS, LOADING BREAK DOWN, DESCRIPTION OF LOADS (I.E. MECH UNIT. SUSPENDED WALL, ETC.) AND THE RATIONALE FOR LOADING DISTRIBUTION ON MULTIPLE MEMBERS. SUBMITTAL SHALL ALSO PROVIDE ANY DOCUMENTATION NECESSARY TO INTERPRET DATA INDICATED ON CALCULATIONS.

NO SCALE

MEMBERS HAVE BEEN DESIGNED TO MEET SERVICEABILITY AND OTHER PERFORMANCE BASED REQUIREMENTS, WHICH MAY EXCEED MINIMUM DESIGN LOADS AND CODE REQUIREMENTS. SUBSTITUTIONS MUST MEET OR EXCEED MOMENT. SHEAR. AND STIFFNESS OF THOSE MEMBERS SPECIFIED AT THE SAME DEPTH AND SPACING.

REFER TO THE <u>FRAMING CONNECTORS</u> SECTION OF THESE GENERAL NOTES FOR REQUIREMENTS PLACED UPON CONNECTOR HARDWARE SPECIFIED BY TRUSS ENGINEER AND/OR PROVIDED BY TRUSS MANUFACTURER.

SPRINKLER LINE ATTACHMENTS SHALL CONFORM TO NFPA 13 AND COMMERCIAL PUBLICATION "SPRINKLER SYSTEM INSTALLATION WITH GUIDELINES FOR REDBUILT OPEN-WEB TRUSSES AND I-JOISTS". LOADS HUNG FROM JOIST NOT SPECIFICALLY IDENTIFIED ON STRUCTURAL DRAWINGS SHALL NOT EXCEED 30 POUNDS AT ANY ONE POINT, NOR SHALL TOTAL LOADS IN POUNDS ON ANY ONE JOIST EXCEED 8 TIMES THE JOIST SPAN IN FEET UNLESS DETAILED OTHERWISE ON THE DRAWINGS. ATTACHMENT OF LOADS EXCEEDING 90 POUNDS SHALL BE APPROVED PRIOR TO INSTALLATION. DO NOT NOTCH OR DRILL THRU TRUSS MEMBERS.

MISCELLANEOUS

PRE-APPROVED SUBSTITUTIONS: SUBSTITUTIONS MAY BE ALLOWED ONLY IF THEY MEET THE REQUIREMENTS OF THESE GENERAL NOTES AND THE SPECIFICATIONS, AND IF COMPLETE WRITTEN ENGINEERING DATA FOR EACH CONDITION REQUIRED FOR THIS PROJECT IS PROVIDED TO THE STRUCTURAL ENGINEER TWO WEEKS PRIOR TO BID DATE AND APPROVED IN WRITTEN ADDENDA BY THE ARCHITECT. DATA IS TO INDICATE CODE BASIS BY YEAR, AUTHORITY FOR STRESSES AND STRESS INCREASES, IF ANY, AND AMOUNT OF EXPECTED DEFLECTION FOR FLEXURAL MEMBERS UNDER (1) TOTAL LOAD AND (2) LIVE LOAD ONLY. ALL INCREASED COSTS IN MECHANICAL, SPRINKLER, ELECTRICAL OR GENERAL INSTALLATION AND ANY ARCHITECTURAL OR STRUCTURAL REDESIGN RESULTING FROM SUBSTITUTION SHALL BE BORNE BY THE GENERAL CONTRACTOR.

SHOP DRAWINGS/SUBMITTALS

THE FOLLOWING SHOP DRAWINGS/SUBMITTALS SHALL BE PROVIDED FOR REVIEW AND APPROVAL BY THE STRUCTURAL ENGINEER PRIOR TO FABRICATION OR DELIVERY.

		STRUCTURAL ENGR.	BLDG. DEPT.
1.	CONCRETE MIX DESIGNS	X	X
2.	REINFORCING STEEL SHOP DRAWINGS	X	
3.	VENEER ANCHORAGE SYSTEMS	X	X
4.	STRUCTURAL STEEL	X	X
5.	GLU-LAMINATED MEMBERS	X	X
6.	STRUCTURAL COMPOSITE LUMBER	X	X
7.	WOOD OPEN-WEB TRUSSES AND I-JOISTS	X	X
8.	METAL-PLATE-CONNECTED WOOD TRUSSES	X	X
9.	CONTRACTOR'S STATEMENT OF RESPONSIBILITY	X	X

DEFERRED SUBMITTALS

THE FOLLOWING ARE NOT INCLUDED WITH THE BUILDING PERMIT DRAWINGS AND SHALL BE SUBMITTED TO THE BUILDING DEPARTMENT AND THE STRUCTURAL ENGINEER FOR REVIEW AND APPROVAL AS A DEFERRED SUBMITTAL SUBMITTALS SHALL BEAR THE SEAL OF AN ENGINEER LICENSED IN THE STATE OF THE PROJECT AS NOTED.

	ENGINEER STAMP REQUIRED
WOOD OPEN-WEB TRUSSES AND I-JOISTS	PE
METAL-PLATE-CONNECTED WOOD TRUSSES	PE

SPECIAL INSPECTION: SPECIAL INSPECTION SHALL BE PROVIDED BY AN INDEPENDENT TESTING LABORATORY PER THE REQUIREMENTS OF IBC CHAPTER 17 AND THE LOCAL BUILDING OFFICIAL OR APPLICABLE JURISDICTION AND THE CONTRACT DOCUMENTS. THE SPECIAL INSPECTOR SHALL SUBMIT INSPECTION REPORTS AND A FINAL SIGNED REPORT TO THE BUILDING OFFICIAL FOR THE ITEMS LISTED IN THE QUALITY ASSURANCE/SPECIAL INSPECTION SECTION:

GENERAL NOTES



STRUCTURAL SYSTEM	VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	COMMENTS	REFERENCES
SOILS	VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY		Х		IBC 1705.6
	VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL		Х		
	PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS		X		
	VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL	X			
	PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY		Х		
CONCRETE	INSPECT REINFORCEMENT, INCLUDING PRE- STRESSING TENDONS, AND VERIFY PLACEMENT		Х		ACI 318: CH 20, 25.2, 25.3, 26.6-1 TO 26.6-3, IBC 1908.4
	ANCHORS CAST IN CONCRETE-PRIOR TO AND DURING PLACEMENT OF CONCRETE		X		ACI 318: 17.8.2 AISC 360 SECTION N7
	ANCHORS POST-INSTALLED IN HARDENED CONCRETE		Х		ACI 318: 3.8.6, 8.1.3, 21.2.8 IBC 1909.1
	VERIFY USE OF REQUIRED DESIGN MIX		Х		ACI 318, CH 19
	PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE	X			ASTM C172, C31 ACI 318: 26.4, 26.12 IBC 1908.10
	MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES		Х		ACI 318: 26.5.3 TO 26.5.5 IBC 1908.9
	MATERIAL VERIFICATION OF REINFORCEMENT STEEL FOR ASTM A615 REINFORCING		Х	MANUFACTURER SHALL PROVIDE MILL TEST REPORTS. CONTINUOUS INSPECTION FOR ALL WELDS GREATER THAN 5/16" FILLET. PERIODIC INSPECTION FOR FILLET WELD 5/16" AND SMALLER	ACI 318: 26.6.4 AWS D1.4 IBC 1705.3.1
	TESTING OF MATERIALS		Х		IBC 1705.3.2
WOOD FRAMING	SHEAR WALL NAILING		Х	SPECIAL INSPECTION NOT REQUIRED FOR FASTENER SPACING > 4" O.C.	IBC 1705.11.1, 1705.12.2, 1705.5
	DIAPHRAGM NAILING		Х	SPECIAL INSPECTION NOT REQUIRED FOR FASTENER SPACING > 4" O.C.	IBC 1705.11.1, 1705.12.2, 1705.5
	NAILING, BOLTING, AND ANCHORAGE OF COMPONENTS THAT ARE PART OF DRAG STRUTS, BRACES AND HOLD-DOWNS THAT ARE PART OF THE SEISMIC RESISTING SYSTEM		Х		IBC 1705.11.1, 1705.12.2
	METAL-PLATE-CONNECTED WOOD TRUSSES SPANNING 60 FEET OR GREATER		Χ	TEMPORARY AND PERMANENT BRACING	IBC 1705.5.2

TESTING AND SPECIAL INSPECTION REPORTS SHALL BE PREPARED FOR EACH INSPECTION ITEM ON A DAILY BASIS WHENEVER WORK IS PERFORMED ON THAT ITEM. REPORTS SHALL BE DISTRIBUTED TO OWNER, CONTRACTOR, BUILDING OFFICIAL, ARCHITECT AND STRUCTURAL ENGINEER OF RECORD.

STRUCTURAL OBSERVATIONS SHALL BE PERFORMED BY THE STRUCTURAL ENGINEER OF RECORD OR DESIGNATED REPRESENTATIVE IN ACCORDANCE WITH IBC 1704.6. STRUCTURAL OBSERVATION SHALL BE PERFORMED AS FOLLOWS:

- » PERIODIC VISUAL OBSERVATION OF STRUCTURAL SYSTEMS FOR GENERAL CONFORMANCE TO CONSTRUCTION DOCUMENTS AT SIGNIFICANT CONSTRUCTION STAGES.
- » REVIEW OF TESTING AND INSPECTION REPORTS.
- » REPORTS SHALL BE PREPARED FOR EACH SITE VISIT AND SHALL BE DISTRIBUTED TO ARCHITECT.

GENERAL CONTRACTOR SHALL SUBMIT A WRITTEN CONTRACTOR'S STATEMENT OF RESPONSIBILITY TO THE BUILDING OFFICIAL AND OWNER PRIOR TO COMMENCEMENT OF WORK. THE CONTRACTOR'S STATEMENT OF RESPONSIBILITY SHALL INCLUDE ACKNOWLEDGMENT OF AWARENESS OF THE SPECIAL INSPECTION REQUIREMENTS CONTAINED IN THE STATEMENT OF SPECIAL INSPECTION.

	ABBREVIAT	ION LIST	
@	AT	HGR	HANGER
A.B.	ANCHOR BOLT	HORIZ.	HORIZONTAL
ADD'L	ADDITIONAL	HSS	HOLLOW STRUCTURAL SECTION
A.F.F.	ABOVE FINISH FLOOR	HT	HEIGHT
ALT.	ALTERNATE	INT.	INTERIOR
ARCH.	ARCHITECTURAL	JST	JOIST
BLD'6	BUILDING	JT	JOINT
BLK'G	BLOCKING	L	ANGLE
BM	BEAM	L.L.	LIVE LOAD
B.O.F.	BOTTOM OF FOOTING	LLH	LONG LEG HORIZONTAL
BOT.	ВОТТОМ	LLV	LONG LEG VERTICAL
BR6	BEARING	LOC.	LOCATION
BTWN	BETWEEN	LSL	LAMINATED STRAND LUMBER
B.V.	BUILT UP	LVL	LAMINATED VENEER LUMBER
(C=)	CAMBER	MAX.	MAXIMUM MAXIMUM
CANT.	CANTILEVER		MACHINE BOLT
CANT.		M.B.	
	COLD-FORMED STEEL	MECH.	MECHANICAL
C.J.	CONTROL/CONSTRUCTION JOINT	MEZZ.	MEZZANINE
<u>Q</u>	CENTERLINE	MFR	MANUFACTURER
CLR.	CLEARANCE	MIN.	MINIMUM
CMU	CONCRETE MASONRY UNIT	MISC.	MISCELLANEOUS
COL.	COLUMN	MTL	METAL
CONC.	CONCRETE	N.F.	NEAR FACE
CONN.	CONNECTION	N.S.	NEAR SIDE
CONST.	CONSTRUCTION	NTS	NOT TO SCALE
CONT.	CONTINUOUS	O.C.	ON CENTER
CONTR.	CONTRACTOR	OPN'G	OPENING
COORD.	COORDINATE	OPP.	OPPOSITE
C.P.	COMPLETE PENETRATION	P.A.F.	POWDER ACTUATED FASTENER
CTR'D	CENTERED	PERP.	PERPENDICULAR
C.Y.	CUBIC YARD	P	PLATE
DBL.	DOUBLE	P.P.	PARTIAL PENETRATION
D.F.	DOUGLAS FIR	P.P.T.	PRESERVATIVE PRESSURE TREATED
DIA. OR Ø	DIAMETER	P.S.F.	POUNDS PER SQUARE FOOT
DIAG.	DIAGONAL	PSL	PARALLAM
DIM.	DIMENSION	P.T.	POST TENSION
D.L.	DEAD LOAD	PW.	PLYWOOD
DWG	DRAWING	REINF.	REINFORCING
DML	DOWEL	REQ'D	REQUIRED
(E)	EXISTING	SCHED.	SCHEDULE
EA.	EACH	S.C.L.	STRUCTURAL COMPOSITE LUMBER
E.F.	EACH FACE	SHT'G	SHEATHING
EL.	ELEVATION	SIM.	SIMILAR
ELEV.	ELEVATOR	5.0.G.	SLAB ON GRADE
ENGR.	ENGINEER	5.0.0.	SQUARE
EQ.	EQUAL	STD	STANDARD
E.W.	EACH WAY	STIFF.	STIFFENER
EXP.	EXPANSION	STL	STEEL
EXT.			
FDN			
ΓνΝ	EXTERIOR	STRUCT.	STRUCTURAL
	EXTERIOR FOUNDATION	STRUCT.	STRUCTURAL TOP & BOTTOM
F.F.	EXTERIOR FOUNDATION FAR FACE	STRUCT. T&B T&G	STRUCTURAL TOP & BOTTOM TONGUE AND GROOVE
FLR	EXTERIOR FOUNDATION FAR FACE FLOOR	STRUCT. T&B T&G THR'D	STRUCTURAL TOP & BOTTOM TONGUE AND GROOVE THREADED
FLR F.O.M.	EXTERIOR FOUNDATION FAR FACE FLOOR FACE OF MASONRY	STRUCT. T&B T&G THR'D T.O.F.	STRUCTURAL TOP & BOTTOM TONGUE AND GROOVE THREADED TOP OF FOOTING
FLR F.O.M. F.O.S.	EXTERIOR FOUNDATION FAR FACE FLOOR FACE OF MASONRY FACE OF STUD	STRUCT. T&B T&G THR'D T.O.F. T.O.S.	STRUCTURAL TOP & BOTTOM TONGUE AND GROOVE THREADED TOP OF FOOTING TOP OF STEEL
FLR F.O.M. F.O.S. FRM'G	EXTERIOR FOUNDATION FAR FACE FLOOR FACE OF MASONRY FACE OF STUD FRAMING	STRUCT. T&B T&G THR'D T.O.F. T.O.S. TRT'D	STRUCTURAL TOP & BOTTOM TONGUE AND GROOVE THREADED TOP OF FOOTING TOP OF STEEL TREATED
FLR F.O.M. F.O.S.	EXTERIOR FOUNDATION FAR FACE FLOOR FACE OF MASONRY FACE OF STUD	STRUCT. T&B T&G THR'D T.O.F. T.O.S.	STRUCTURAL TOP & BOTTOM TONGUE AND GROOVE THREADED TOP OF FOOTING TOP OF STEEL TREATED TYPICAL
FLR F.O.M. F.O.S. FRM'G	EXTERIOR FOUNDATION FAR FACE FLOOR FACE OF MASONRY FACE OF STUD FRAMING	STRUCT. T&B T&G THR'D T.O.F. T.O.S. TRT'D	STRUCTURAL TOP & BOTTOM TONGUE AND GROOVE THREADED TOP OF FOOTING TOP OF STEEL TREATED
FLR F.O.M. F.O.S. FRM'G F.R.T.	EXTERIOR FOUNDATION FAR FACE FLOOR FACE OF MASONRY FACE OF STUD FRAMING FIRE RETARDANT TREATED	STRUCT. T&B T&G THR'D T.O.F. T.O.S. TRT'D TYP.	STRUCTURAL TOP & BOTTOM TONGUE AND GROOVE THREADED TOP OF FOOTING TOP OF STEEL TREATED TYPICAL
FLR F.O.M. F.O.S. FRM'G F.R.T. F.S.	EXTERIOR FOUNDATION FAR FACE FLOOR FACE OF MASONRY FACE OF STUD FRAMING FIRE RETARDANT TREATED FAR SIDE	STRUCT. T&B T&G THR'D T.O.F. T.O.S. TRT'D TYP. U.N.O.	STRUCTURAL TOP & BOTTOM TONGUE AND GROOVE THREADED TOP OF FOOTING TOP OF STEEL TREATED TYPICAL UNLESS NOTED OTHERWISE
FLR F.O.M. F.O.S. FRM'G F.R.T. F.S. FTG	EXTERIOR FOUNDATION FAR FACE FLOOR FACE OF MASONRY FACE OF STUD FRAMING FIRE RETARDANT TREATED FAR SIDE FOOTING	STRUCT. T&B T&G THR'D T.O.F. T.O.S. TRT'D TYP. U.N.O. U.T.	STRUCTURAL TOP & BOTTOM TONGUE AND GROOVE THREADED TOP OF FOOTING TOP OF STEEL TREATED TYPICAL UNLESS NOTED OTHERWISE ULTRASONIC TESTED
FLR F.O.M. F.O.S. FRM'G F.R.T. F.S. FTG GA.	EXTERIOR FOUNDATION FAR FACE FLOOR FACE OF MASONRY FACE OF STUD FRAMING FIRE RETARDANT TREATED FAR SIDE FOOTING GAGE/GAUGE	STRUCT. T&B T&G THR'D T.O.F. T.O.S. TRT'D TYP. U.N.O. U.T. VERT.	STRUCTURAL TOP & BOTTOM TONGUE AND GROOVE THREADED TOP OF FOOTING TOP OF STEEL TREATED TYPICAL UNLESS NOTED OTHERWISE ULTRASONIC TESTED VERTICAL
FLR F.O.M. F.O.S. FRM'G F.R.T. F.S. FTG GA. GALV.	EXTERIOR FOUNDATION FAR FACE FLOOR FACE OF MASONRY FACE OF STUD FRAMING FIRE RETARDANT TREATED FAR SIDE FOOTING GAGE/GAUGE GALVANIZED	STRUCT. T&B T&G THR'D T.O.F. T.O.S. TRT'D TYP. U.N.O. U.T. VERT. W/	STRUCTURAL TOP & BOTTOM TONGUE AND GROOVE THREADED TOP OF FOOTING TOP OF STEEL TREATED TYPICAL UNLESS NOTED OTHERWISE ULTRASONIC TESTED VERTICAL WITH
FLR F.O.M. F.O.S. FRM'G F.R.T. F.S. FTG GA. GALV. GL.	EXTERIOR FOUNDATION FAR FACE FLOOR FACE OF MASONRY FACE OF STUD FRAMING FIRE RETARDANT TREATED FAR SIDE FOOTING GAGE/GAUGE GALVANIZED GLULAM	STRUCT. T	STRUCTURAL TOP & BOTTOM TONGUE AND GROOVE THREADED TOP OF FOOTING TOP OF STEEL TREATED TYPICAL UNLESS NOTED OTHERWISE ULTRASONIC TESTED VERTICAL WITH WORK POINT





Archdiocese of Seattle St. Andrew Catholic Church Parish 1401 Valley Avenue East Sumner, WA 98390-2720

 Job No:
 1851
 Date:
 2/7/2020

 File No:
 19-232

 Drawn By:
 MF

 Checked By:
 DRS

 Issued For:
 PERMIT

GENERAL NOTES

S004

INDICATES CONCRETE STEM WALL.

INDICATES WOOD STUD BUILT-UP COLUMNS. SEE 2/S3Ø1 FOR DETAIL.

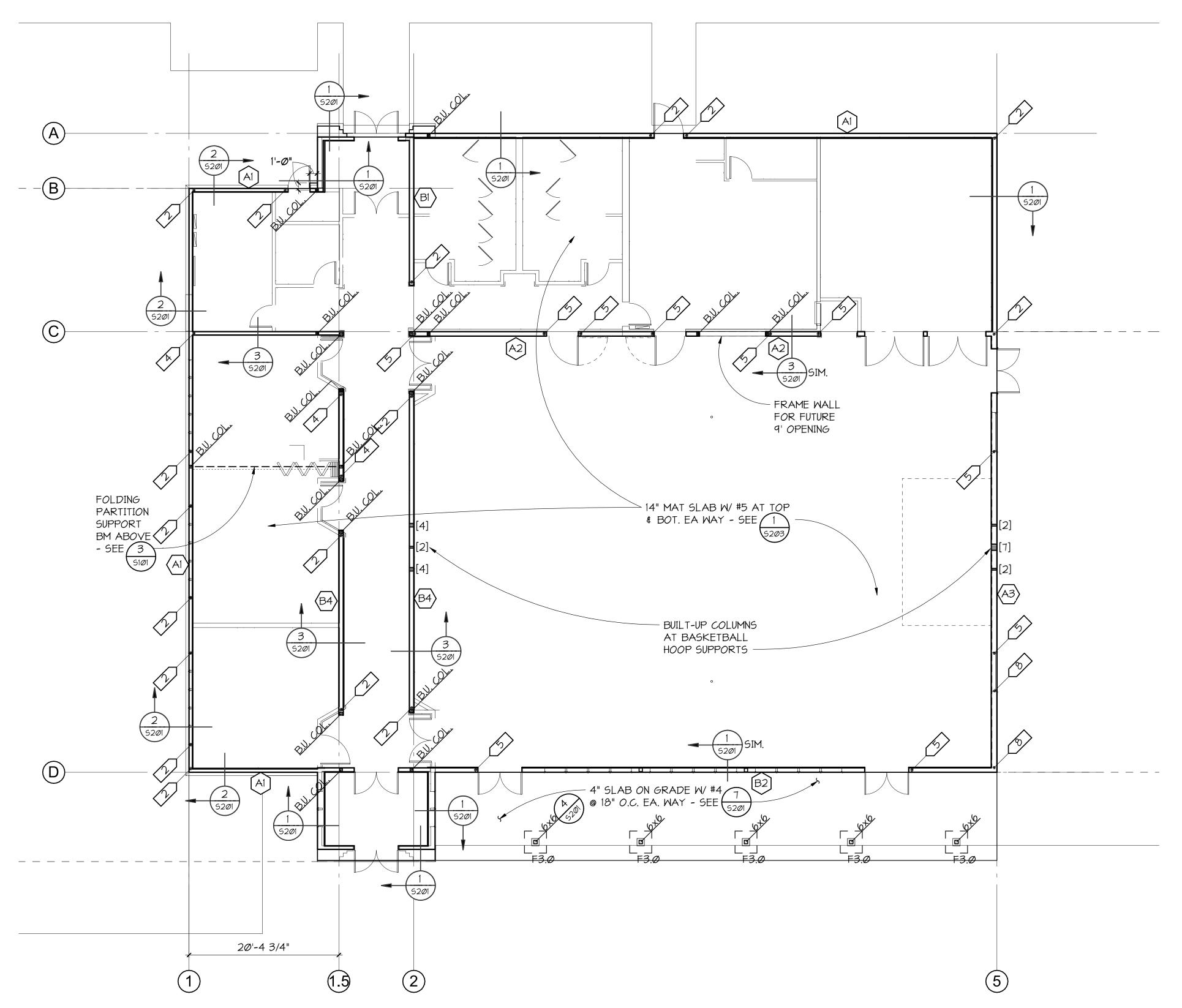
5. FOR TYPICAL FOUNDATION DETAILS SEE SHEET S201.

INDICATES SPECIAL BUILT-UP WOOD STUD COLUMN REQUIREMENTS UNDER HEADER. FOR TYPICAL FRAMING REQUIREMENTS AT OPENING IN STRUCTURAL WALLS SEE 1/S3Ø1 FOR TYPICAL DETAIL.

INDICATES SPECIAL WOOD STUD WALL TYPE. SEE 4/5301 FOR SCHEDULE.

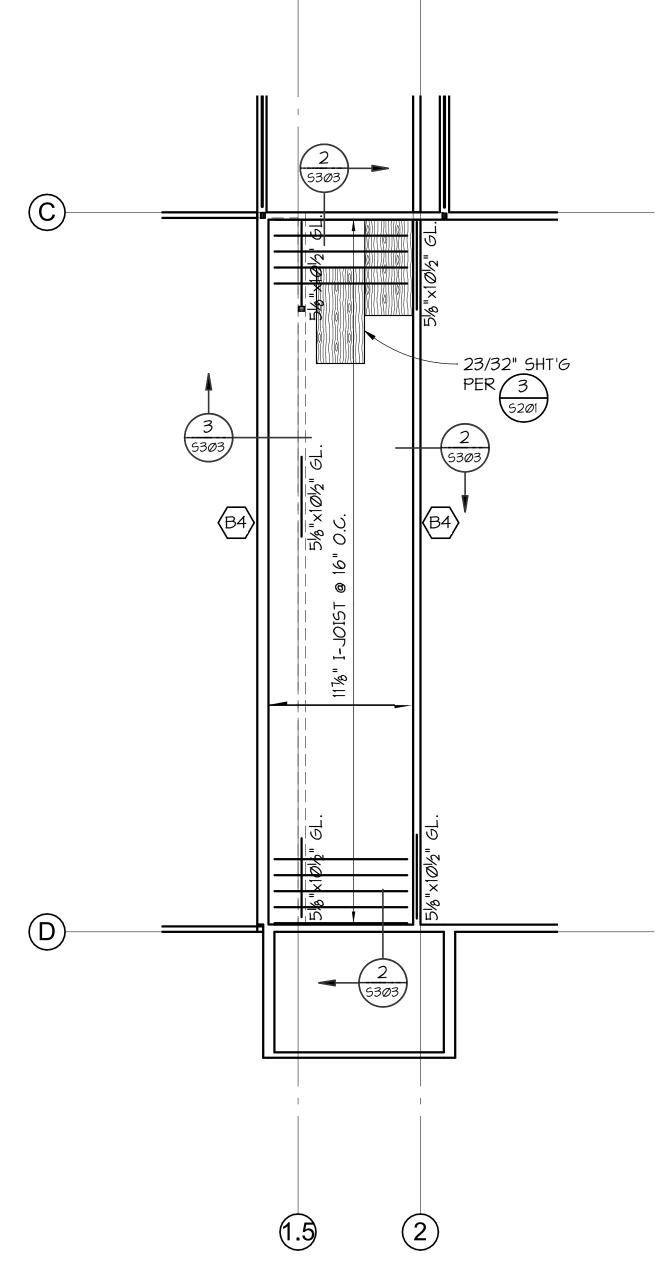
INDICATES HOLDOWN. SEE 4/S3Ø1 FOR SCHEDULE.

9. NON-STRUCTURAL STUD WALLS ARE NOT SHOWN OR SHOWN SCREENED. FOR LOCATION SEE ARCHITECTURAL DRAWINGS. FOR BRACING AT TOPS OF WALLS SEE S304.



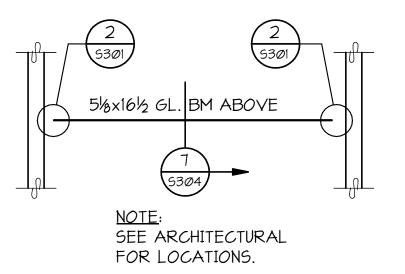






MECHANICAL PLATFORM FRAMING PLAN

| SIØI | 1/8" = 1'-Ø"



TYPICAL FOLDING WALL SUPPORT BEAM





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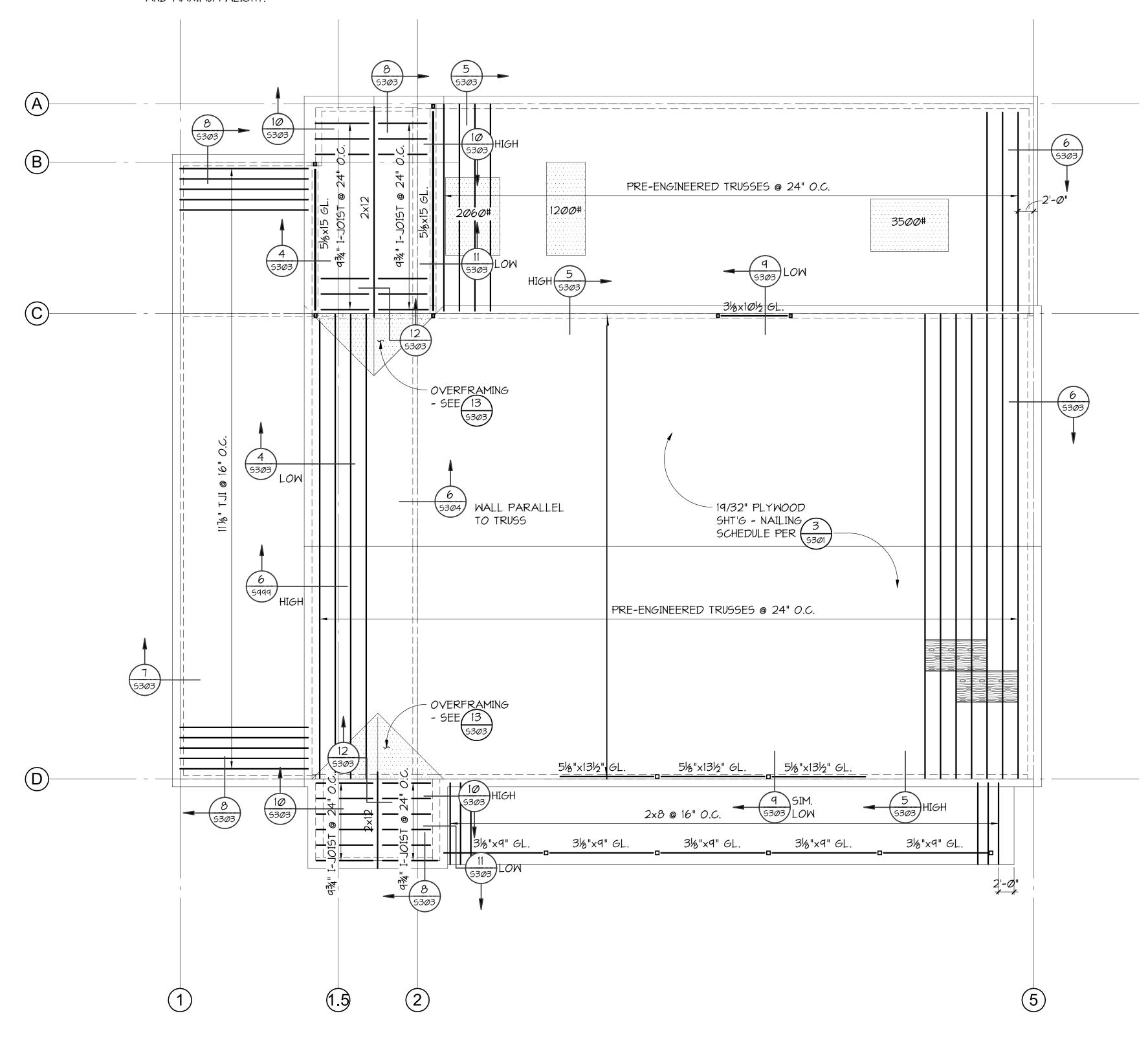
Drawn By: Checked By: DRS

FOUNDATION PLAN

- INDICATES TYPICAL HEADER IN WALL BELOW. SEE 1/53Ø1.

4. PROVIDE 19/32" WOOD SHEATHING OVER ENTIRE ROOF STRUCTURE. NAIL SHEATHING WITH 10d @ 6" ON CENTER AT ALL SUPPORTED PANEL EDGES AND 100 0 10" ON CENTER AT INTERMEDIATE FRAMING. TYPICAL UNLESS NOTED OTHERWISE.

INDICATES LOCATION OF ROOF TOP MECHANICAL UNITS AND MAXIMUM WEIGHT.









Jule Jatholic East -2720

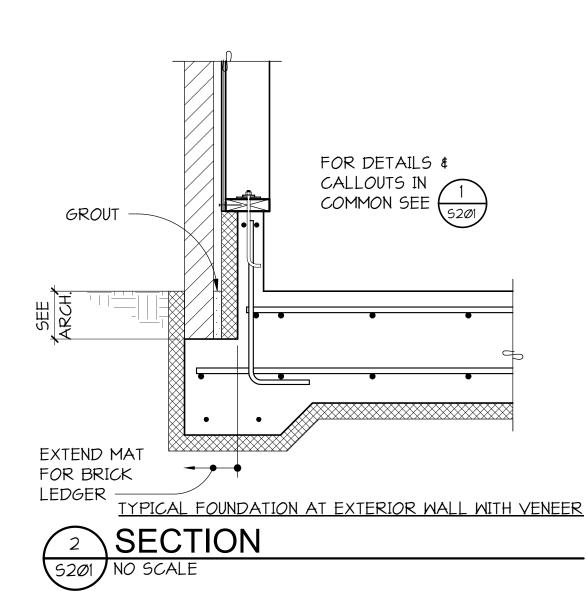
1851 Date: 2/7/2020 Drawn By: Checked By:

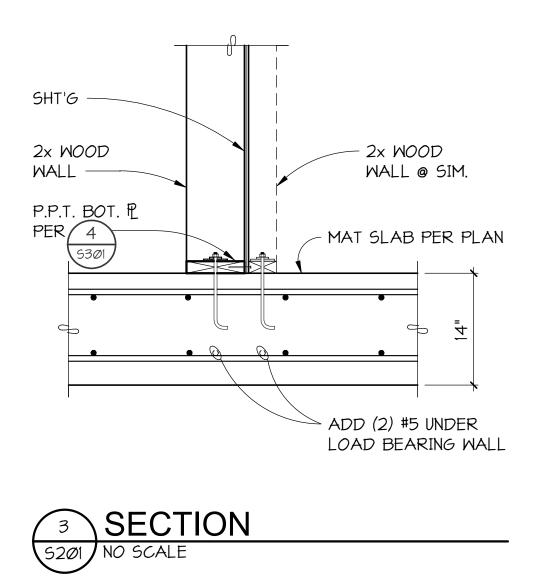
ROOF FRAMING PLAN

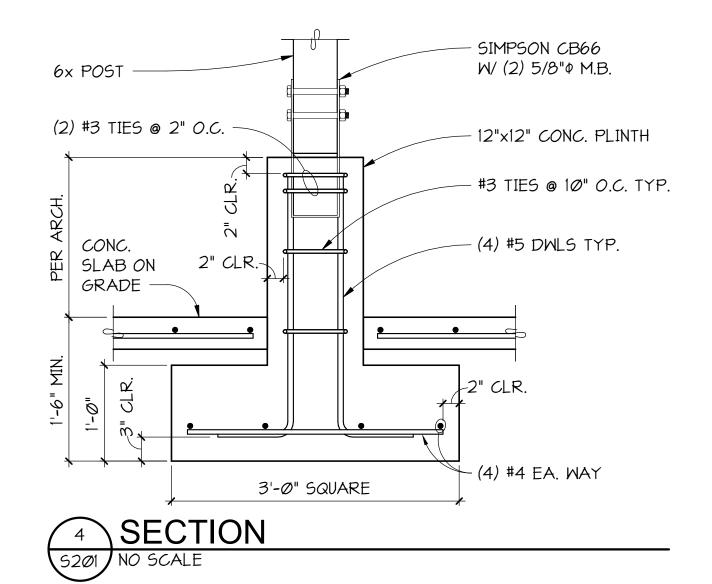
Structural Solutions
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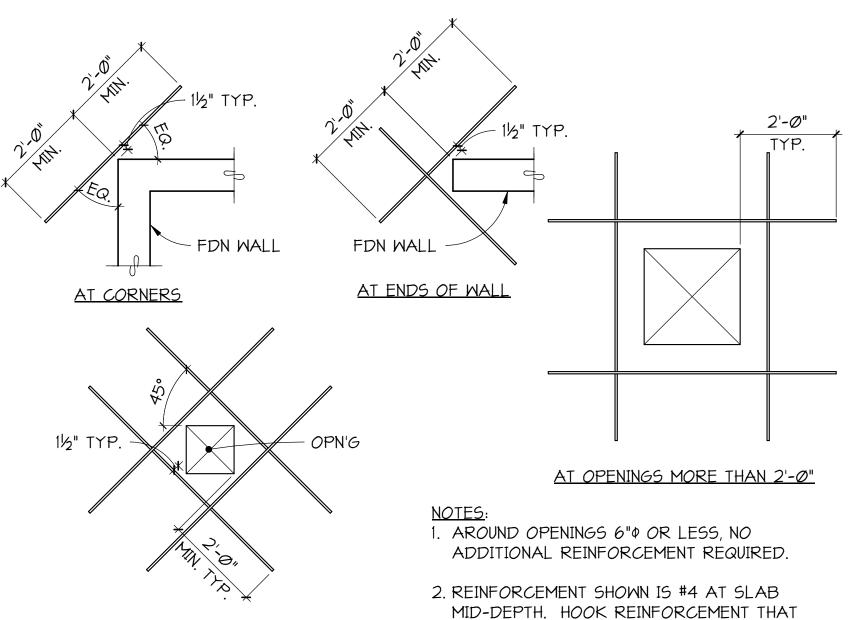
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GRID

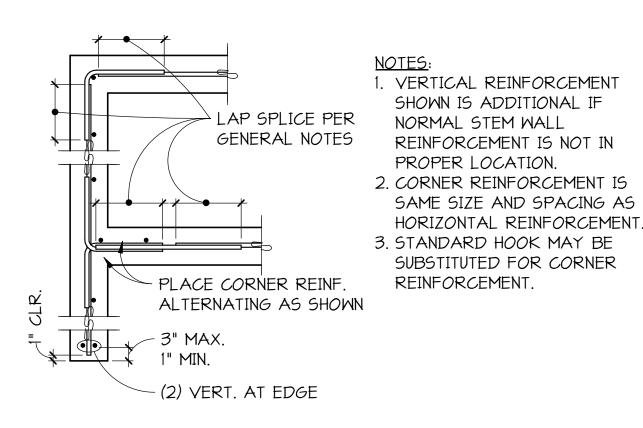


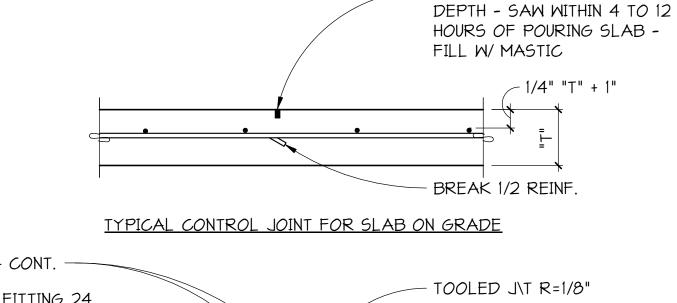




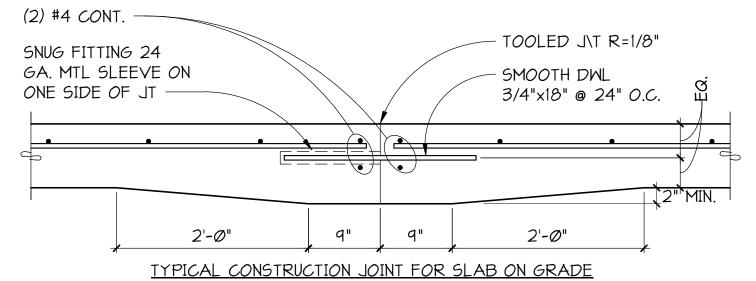


CANNOT BE EXTENDED.





- 1/8" SAWCUT x 1/4 SLAB





TYPICAL SLAB ON GRADE DISCONTINUITY REINFORCEMENT



AT OPENINGS LESS THAN 2'-0"





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1851 Date: 2/7/2020 Drawn By: Checked By:

> **FOUNDATION DETAILS**



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DBL. TOP PL W/ LAP

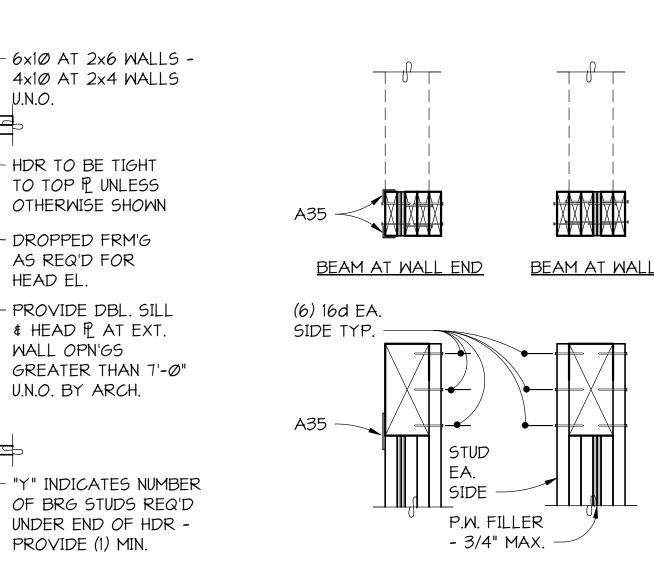
PER 3/WSTD3 -

(6) 16d

TYP. -

(4) 16d

TYP.



TYPICAL BUILT-UP COLUMN AT BEAM PERPENDICULAR TO WALL

DETAIL S3Ø1 NO SCALE

STUD WALL CONSTRUCTION SCHEDULE										
	TABLE 1 - SHEAR WALL REQUIREMENTS									
MARK	MALL SHEATHING	SIDES WITH SHEATHING	SHEATHING NAILS NOTE 2	EDGE NAILING ON CENTER	EDGE FRAMING NOTE 5	FIELD NAILING ON CENTER	BOTTOM PLATE NOTE 6	BOTTOM PLATE NAILING	5/8" ANCHOR BOLT SPACING (EMBED 7" MINIMUM)	RIM/BLOCKING CONNECTOR TO TOP PLATE BELOW
A	15/32"	(1)	1Ød	6"	2x	12"	2x	16d @ 8" O.C.	48"	A35 @ 24" O.C.
B	15/32"	(1)	1Ød	4"	3x	12"	2x	16d @ 8" O.C.	32"	A35 @ 16" O.C.
T	 ABLE 2 - ST	UD REQUIRE	MENTS		EtDC:		D INDICATI			<u> </u>

TABLE 2 - STUD REQUIREMENTS						
MARK	STUD SIZE AND SPACING	NUMBER STUDS REQUIRED AT MEMBER BEARING				
1	2×6 @ 16" O.C.	1				
$\langle 2 \rangle$	2x8 @ 12" O.C.	2				
(3)	1½×9¼ LVL @ 16" O.C.	1				
4	1½×7¼ LVL @ 16" O.C.	1				

- A35 TYP. OR

A34 AT 2x4

"X" INDICATES NUMBER OF

ADJACENT TO END OF HDR

FULL HEIGHT STUDS

PROVIDE (1) MIN.

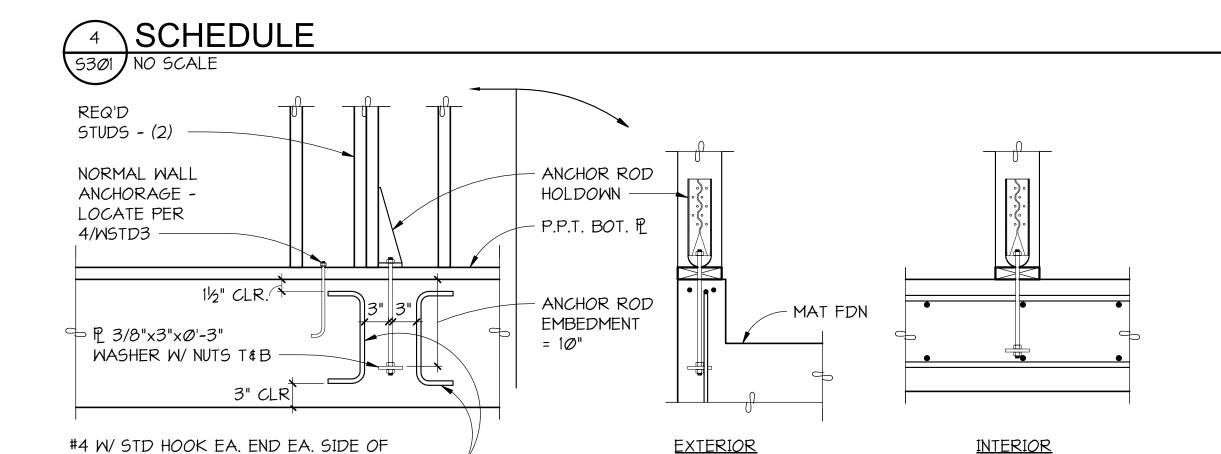
WALLS

FIRST CHARACTER INDICATES SPECIAL SHEAR WALL

SECOND CHARACTER INDICATES SPECIAL STUD SPACING PER TABLE 2

(xx)INDICATES SPECIAL STRUCTURAL WALL MARK. ALL WALLS SHOWN ON STRUCTURAL DRAWINGS ARE 2x6 AT 16" ON CENTER UNLESS DESIGNATED SPECIAL. STUD LAYOUT SHALL MATCH FRAMING MEMBER LAYOUT ABOVE WHERE APPLICABLE. ALL EXTERIOR WALLS SHALL HAVE 15/32" WOOD SHEATHING BE NAILED WITH 10d AT 6" ON CENTER AT EDGES AND 12" ON CENTER IN FIELD UNLESS DESIGNATED SPECIAL.

- 2. ALL EXTERIOR WALLS AND ALL DESIGNATED SHEAR WALLS SHALL BE BLOCKED AT ALL SHEATHING EDGES. EDGE NAILING APPLIES TO ALL TOP AND BOTTOM PLATES, VERTICAL JOINTS, HORIZONTAL BLOCKED JOINTS, WALL CORNERS, AND HOLDOWN ANCHORED STUDS.
- 3. WHERE BEAMS OR HEADERS FRAME INTO WALLS AND A COLUMN IS NOT CALLED OUT, PROVIDE BUILT-UP COLUMNS PER 4/WSTD2 FOR BEAM PERPENDICULAR TO WALL.
- 4. [XYINDICATES BUILT-UP STUD COLUMNS AT HEADERS IN WALLS SEE 1/WSTD2 FOR BEAM PARALLEL TO WALL. 5. PROVIDE 3x OR DOUBLE 2x MEMBERS FACE NAILED PER 1/WSTD2 AT ALL ABUTTING PANEL EDGES WHERE INDICATED. 6. 3x BOTTOM PLATE WHERE INDICATED.
- 7. WHERE SOLID SAWN STUD LENGTH CANNOT BE OBTAINED, STRUCTURAL COMPOSITE LUMBER STUDS MAY BE SUBSTITUTED. SOLID SAWN FRAMING MAY NOT BE SUBSTITUTED FOR SPECIFIED STRUCTURAL COMPOSITE LUMBER FRAMING.



TYPICAL FOUNDATION ANCHOR ROD HOLDOWN

ANCHOR ROD - TOTAL QUANTITY PER SCHED.



DIAPHRAGM NAILING SCHEDULE					
DIAPHRAGM TYPE	LOCATION	NAILS	SPACING		
FLOOR DIAPHRAGM 23/32" TONGUE	DIAPHRAGM BOUNDARY	1Ød	6" O.C.		
AND GROOVE SHEATHING UNBLOCKED	FIELD NAILS	1Ød	12" O.C.		
UNLESS NOTED OTHERWISE	SUPPORTED PANEL EDGES	1Ød	6" O.C.		
ROOF DIAPHRAGM	DIAPHRAGM BOUNDARY	1Ød	6" O.C.		
19/32" SHEATHING UNBLOCKED	FIELD NAILS	1Ød	10" O.C.		
UNLESS NOTED OTHERWISE	SUPPORTED PANEL EDGES	1Ød	6" O.C.		

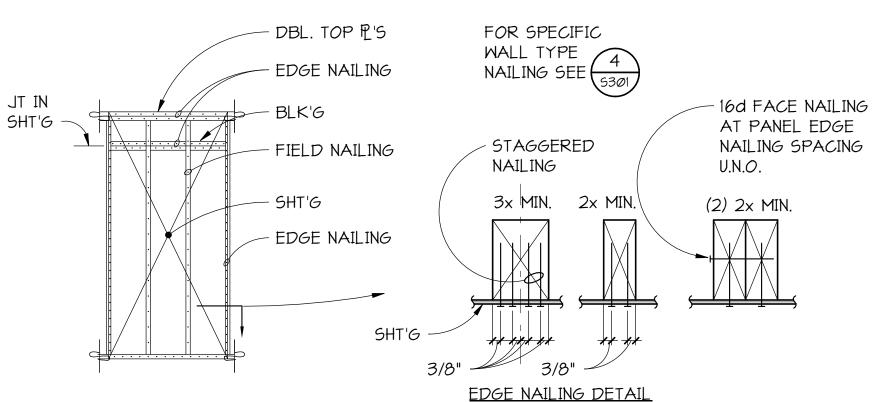
NOTES:

BEAM AT HIP

- 1. PROVIDE (2) ROWS OF SPECIFIED DIAPHRAGM BOUNDARY NAILING OVER INTERIOR SHEAR WALLS AND THE FULL LENGTH OF "COLLECTORS" WHERE INDICATED.
- 2. AT BLOCKED DIAPHRAGMS PROVIDE 2x4 FLATWISE BLOCKING WITH "Z2" CLIPS AT EACH END AT ALL UNSUPPORTED PANEL EDGES. USE 2x4 STRUCTURAL COMPOSITE LUMBER FLATWISE BLOCKING IN LIEU OF SOLID SAWN WHERE NAILING SIZE OR SPACING EXCEEDS 10d @ 4" O.C.

TYPICAL DIAPHRAGM NAILING

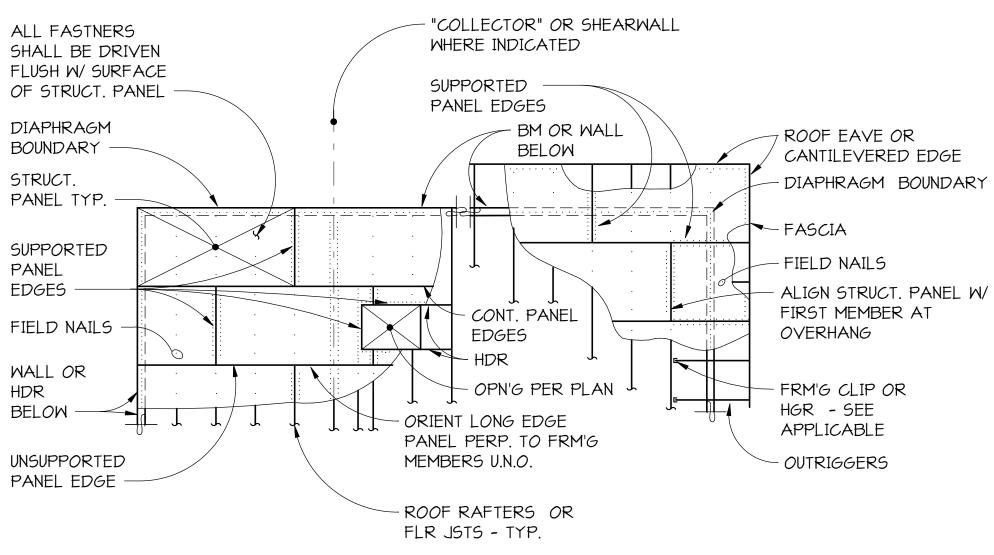


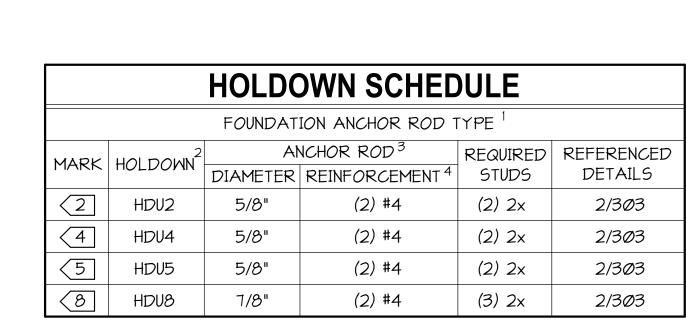


- 1. PANEL EDGE NAILING AND PLATE NAILING SHALL BE STAGGERED IN ALL CASES.
- 2. SHEATHING JOINT SHALL OCCUR AT COMMON MEMBER UNLESS IT OCCURS AT A SPECIFIED DOUBLE MEMBER.
- 3. EDGE NAILING APPLIES TO AREAS INDICATED AND AT HOLDOWN ANCHORED STUDS.

TYPICAL SHEARWALL NAILING



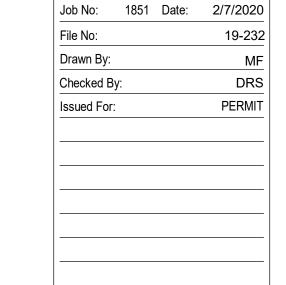




NOTES:

- 1. ALL HOLDOWNS SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS.
- 2. PROVIDE BACK TO BACK ANCHOR ROD HOLDOWNS ACROSS FLOOR LINE PER 5/WSTD4.
- 3. ALL-THREAD ROD ASTM A36 WITH 3"x3"x3/8" PLATE WITH DOUBLE NUTS AT FOUNDATION.
- 4. EMBEDMENT MAY REQUIRE STEPPING DOWN FOOTING PER 1/CFØ1 TO ACHIEVE REQUIRED EMBEDMENT.





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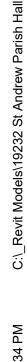
of

Archdioce

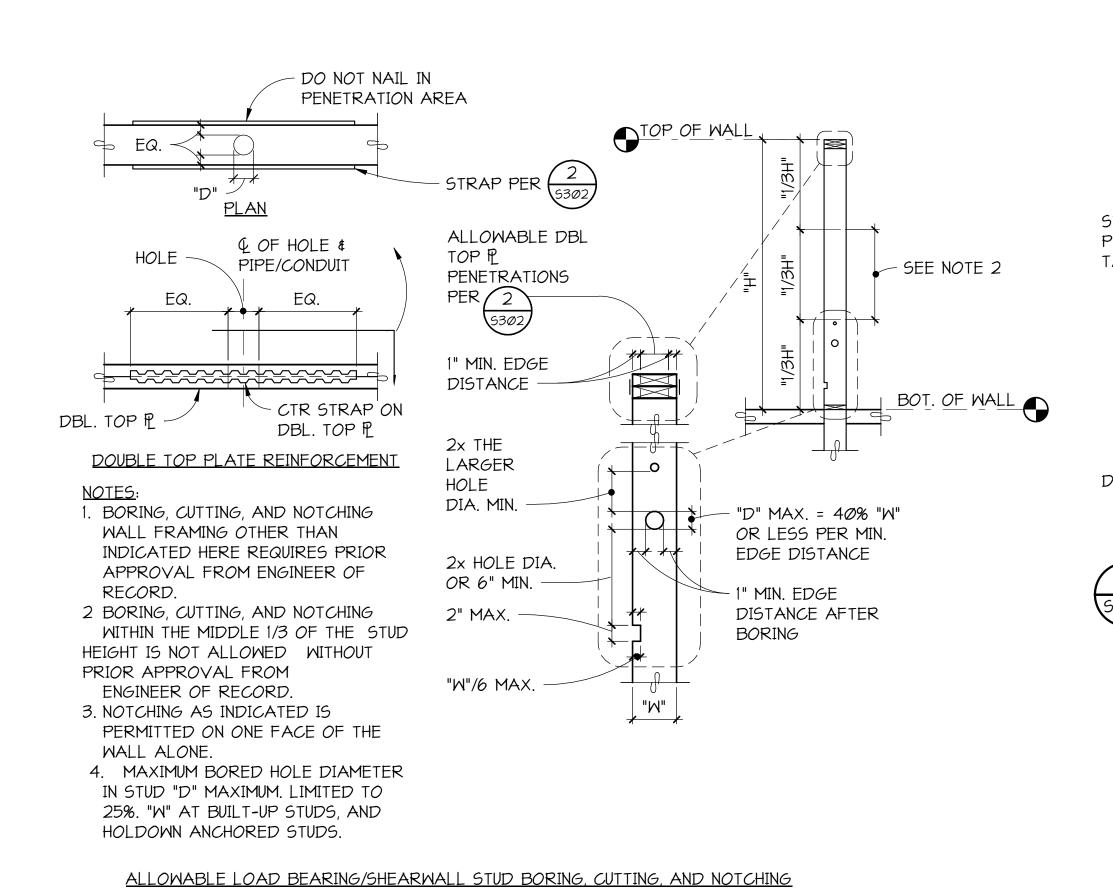
FRAMING DETAILS

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BPS BRG PW/

SLOTTED HOLE

UNDER STD CUT

√1/2" MAX.

WASHER

WASHER & NUT

A.B. W/ 7" MIN. EMBED TYP. SEE

STUD WALL CONST. SCHED. FOR

1/2" MAX. TO

BPS BRG PS W/

SLOTTED HOLE UNDER STD CUT

WASHER & NUT

SHT'G

SIZES & SPACINGS

DETAIL

DIAMETER.

SHEAR WALLS.

- A MIN. OF (2) A.B.

SHALL BE PROVIDED IN EA. BOT. P. SECTION/

1. BOTTOM SILL PLATE SHALL BE PRESERVATIVE

NOTES FOR GALVANIZED REQUIREMENTS FOR

TO A MAXIMUM OF 1/16" LARGER THAN THE BOLT

PRESSURE PLATES SHALL BE TREATED WITH A 9%

CONNECTORS AND FASTENERS.

SOLUTION OF COPPER NAPHTHENATE.

PRESSURVATIVE PRESSURE TREATED. SEE GENERAL

2. HOLES IN BOTTOM PLATE SHALL BE A MINIMUM OF 1/32"

3. HOLES, CUTS, AND NOTCHES IN 3x OR 4x PRESERVATIVE

OVERSIZE WASHERS MAY BE USED IN LIEU OF BPS.

STAGGER ANCHOR BOLTS AT DOUBLE SHEATHED

5302 NO SCALE

"L"=6" MIN. \$ 12" MAX.

DETAIL

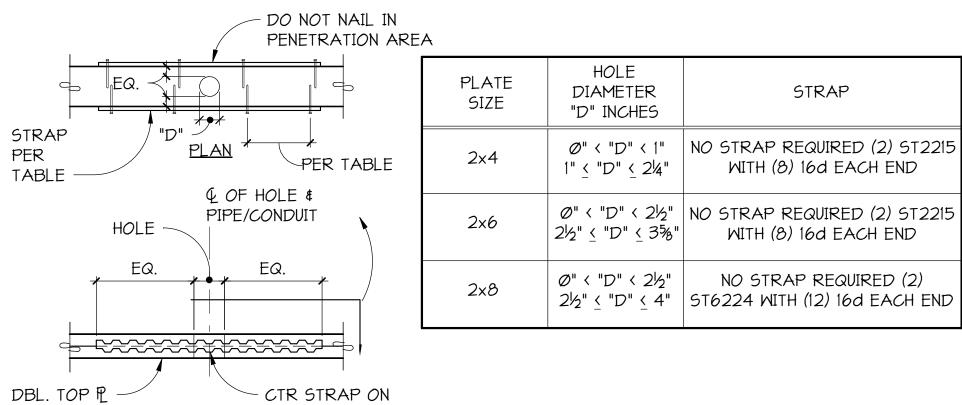
5302 NO SCALE

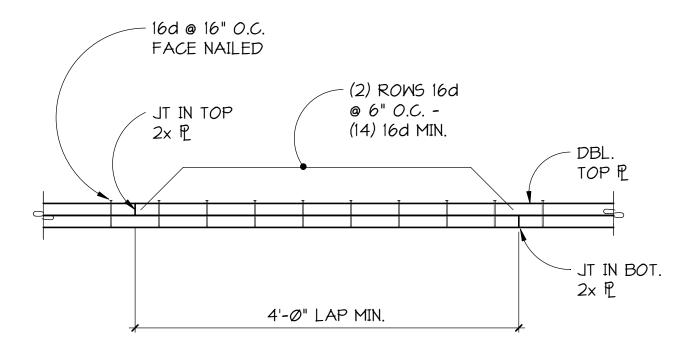
PROVIDE (1) A.B. EA. SIDE WHERE

NOTCHED MORE THAN 1/3 THE P

TYPICAL BOTTOM PLATE ANCHORAGE

BOT. P. IS BORED, CUT OR



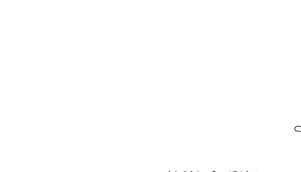


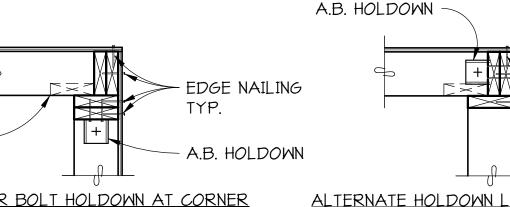
3 SECTION S302 NO SCALE

TYPICAL REINFORCING AT WALL DOUBLE TOP PLATE PENETRATIONS

DBL. TOP P

DETAIL S302 NO SCALE



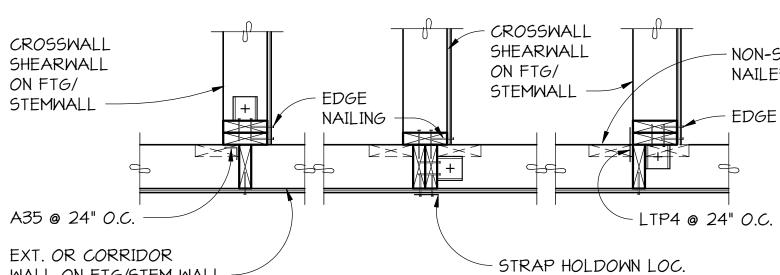


ALTERNATE HOLDOWN LOCATION AT CORNER

NON-STRUCT.

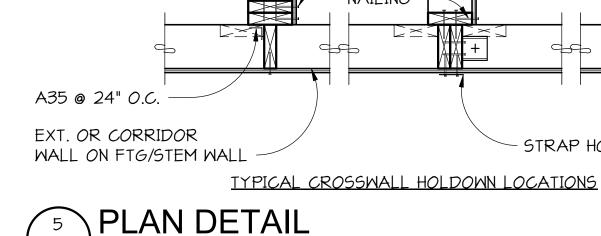
NAILER - TYP.

EDGE NAILING





- EDGE NAILING NON-STRUCT. NAILER - TYP. TYP. ANCHOR BOLT HOLDOWN AT CORNER



19-232 Drawn By: DRS Checked By: PERMIT FRAMING DETAILS

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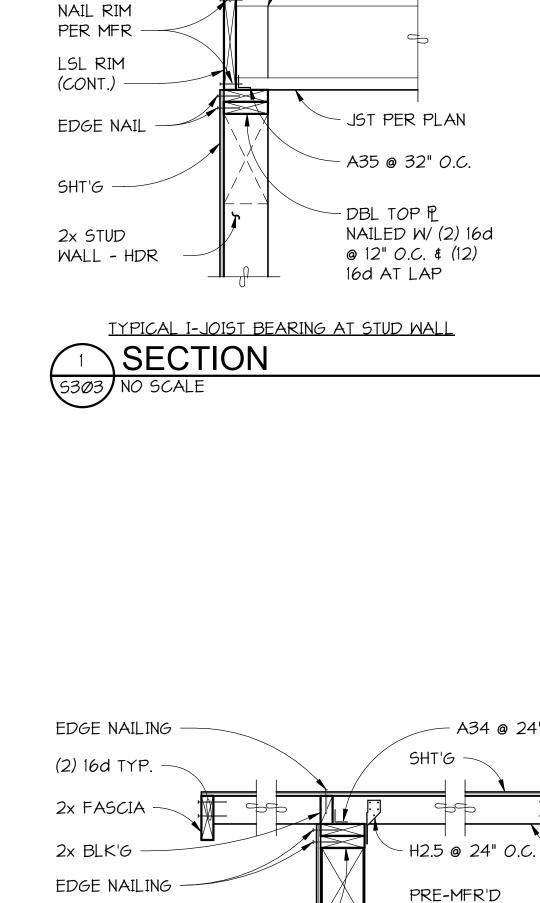
And

1851 Date: 2/7/2020

Seattle

of

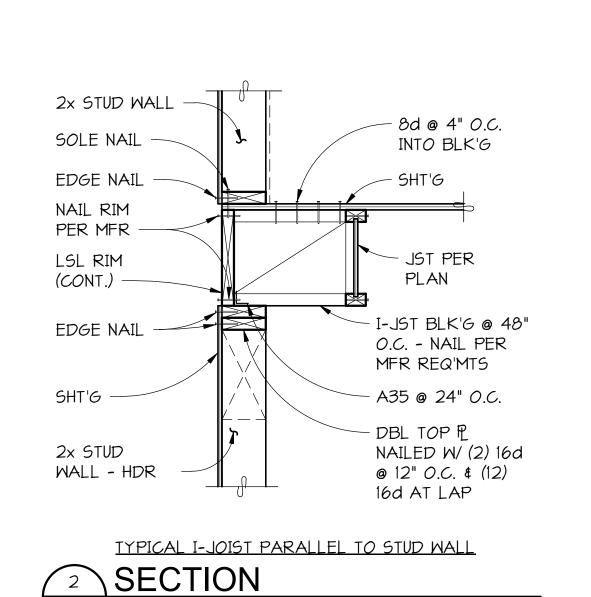




2x STUD WALL

SOLE NAIL -

EDGE NAIL



S303 NO SCALE

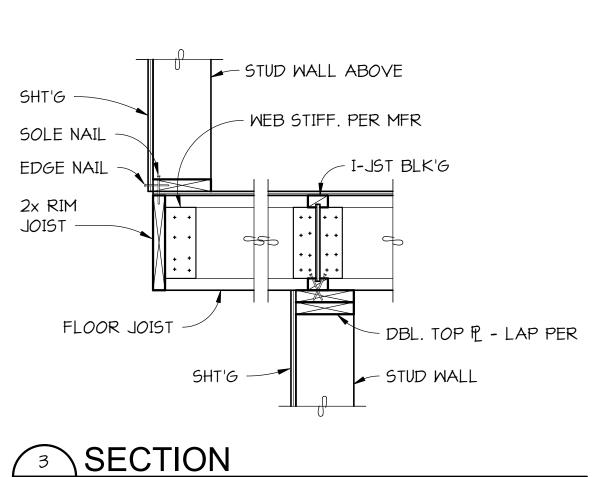
(2) 16d TYP.

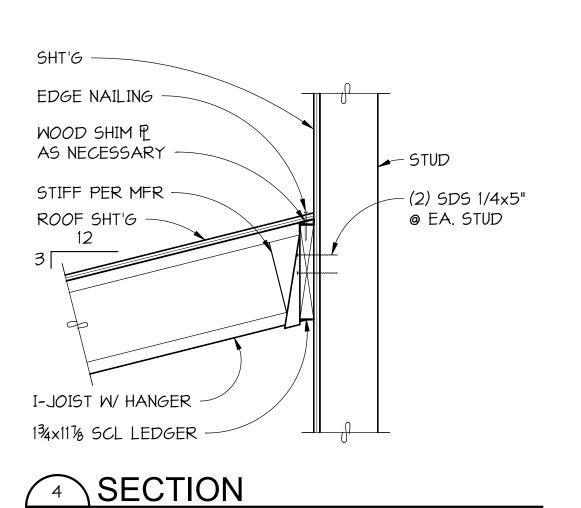
- 2x4 RAKE

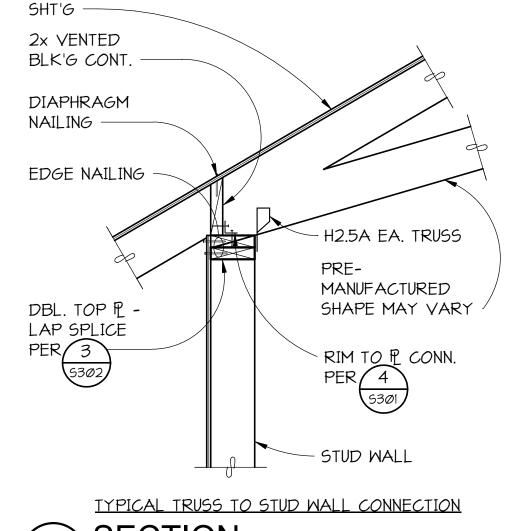
RAFTER @

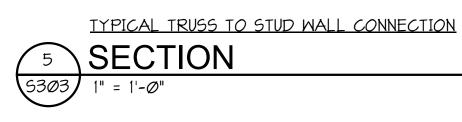
PER PLAN

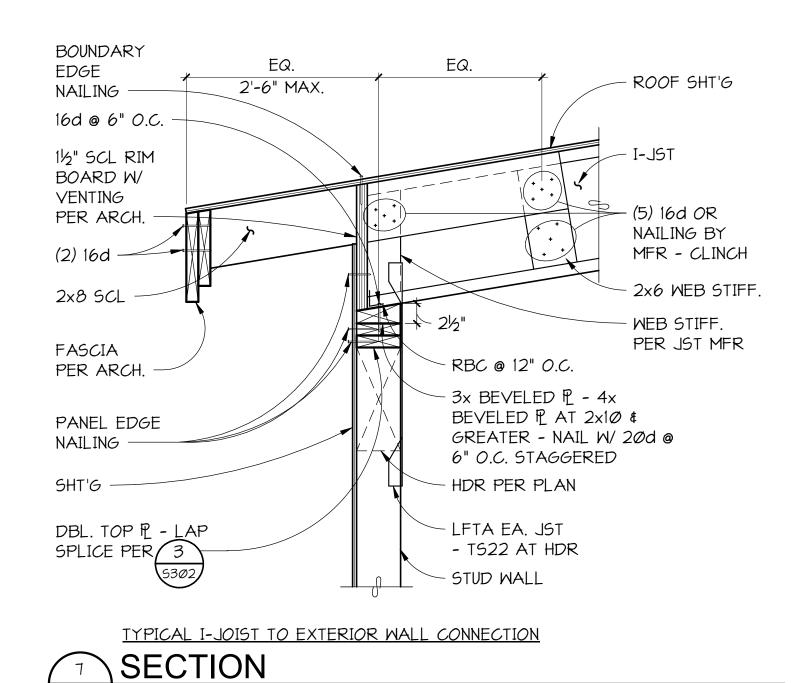
24" O.C.



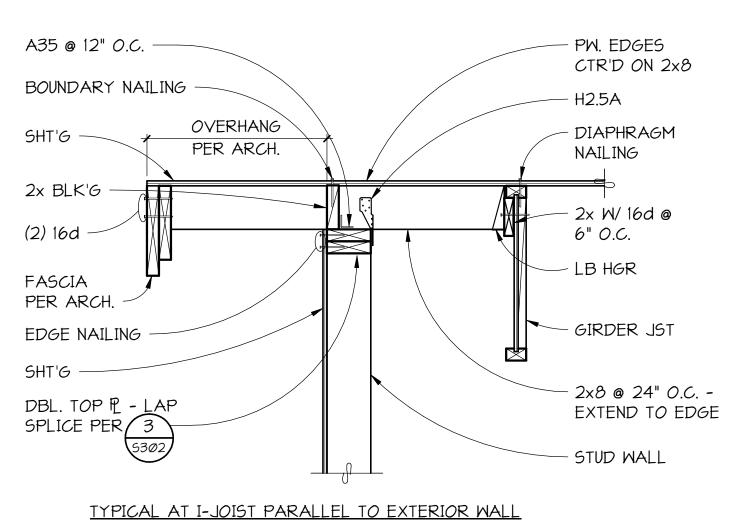




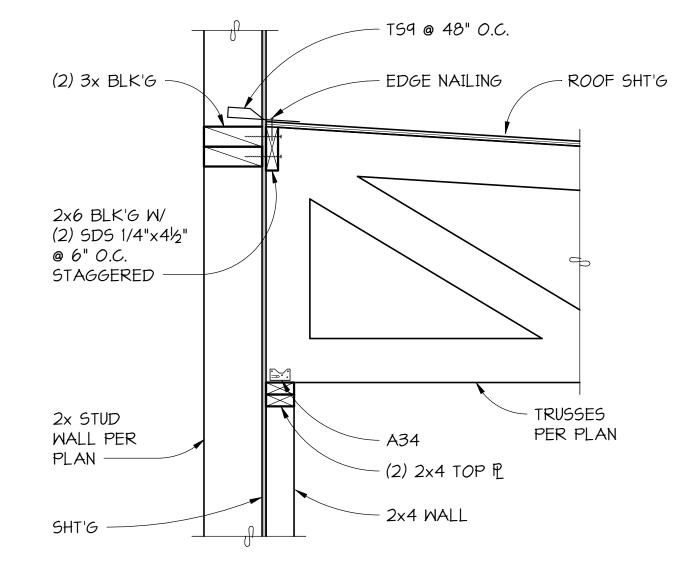


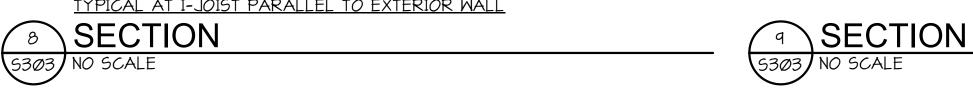


S3Ø3 NO SCALE



S3Ø3 NO SCALE







TRUSS PER

2x STUD WALL

HDR AT SIM.

PLAN -

- A34 @ 24" O.C.

NOTE: AT 9½" I-JOIST, PROVIDE 2¾" x 5½" P

ON TOP OF DBL. TOP P - FASTEN WITH 200d

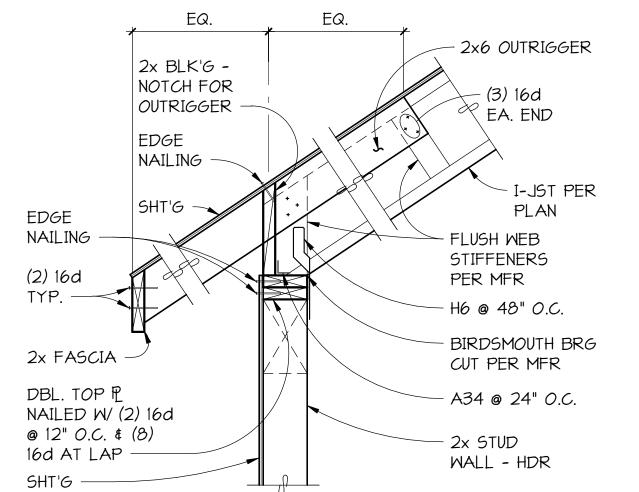
WEB STIFF. PER MFR

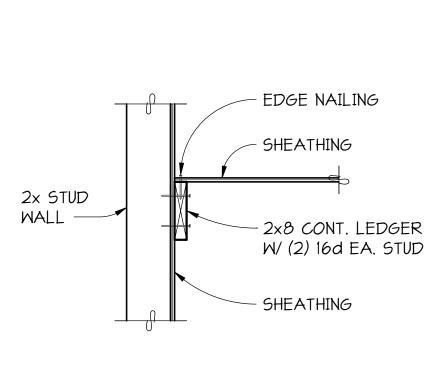
@ 6" O.C.

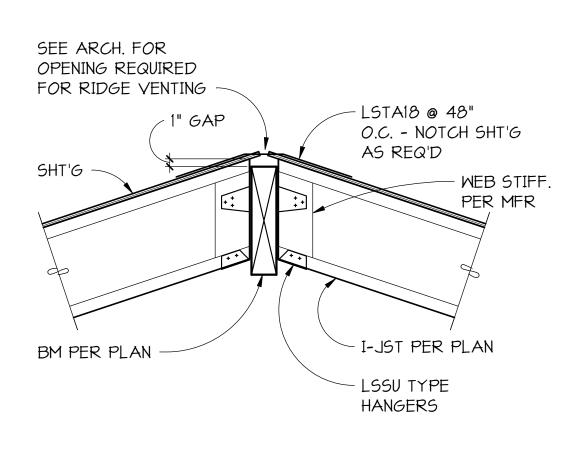


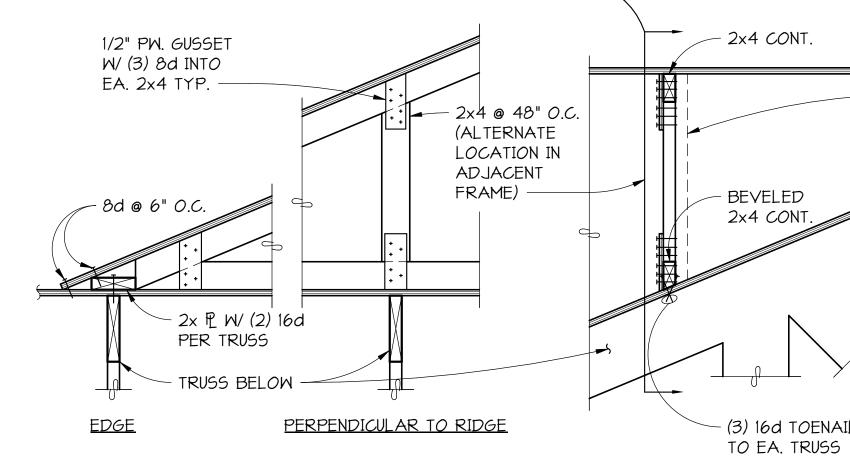
DBL TOP PL NAILED W/ (2) 16d @ 12"

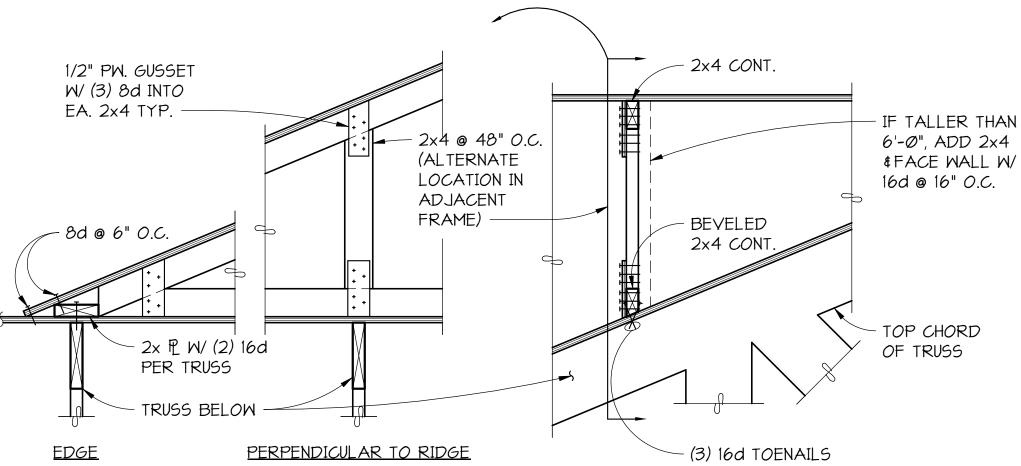
\$ (8) 16d AT LAP





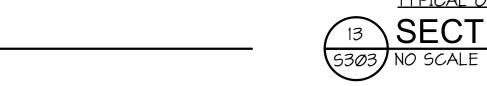






PARALLEL TO RIDGE

TYPICAL OVERLAY FRAMING





Archdiocese h 1401 Sumr 1851 Date: 2/7/2020 Drawn By: DRS Checked By: Issued For:

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Seattle

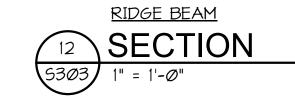
of

FRAMING DETAILS

I-JOIST BEARING AT STUD WALL SECTION

5303 1" = 1'-0"





2. "L" INDICATES UNBRACED LENGTH OF WALLS. 3. AT CONTRACTORS OPTION IN LIEU OF EXTRA

(1) INDICATES HORIZONTAL BRACE EXTENDING TO ADJACENT CORNER SEE 2/5304.

(2) INDICATES BRACE UP TO ROOF STRUCTURE SEE 3/S3Ø4.

TYPICAL LATERAL SUPPORT FOR INTERIOR NON-BEARING WALLS NOT

DETAIL S304 NO SCALE

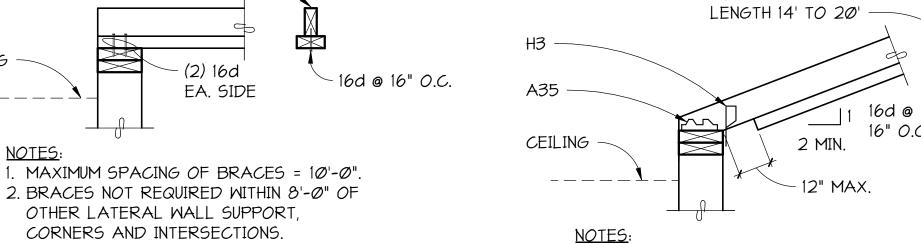
TOP PLATE.

LENGTH 14' TO 20' CEILING - (2) 16d EA. SIDE ______ 1. MAXIMUM SPACING OF BRACES = 10'-0".

(2) 2x4 FOR BRACE

LENGTH UP TO 14' -

(2) 2x6 FOR BRACE



(2) 2x4 FOR BRACE

LENGTH UP TO 14' -

(2) 2x6 FOR BRACE

1. MAXIMUM SPACING OF BRACES = 10'-0". 2. BRACES NOT REQUIRED WITHIN 8'-0" OF

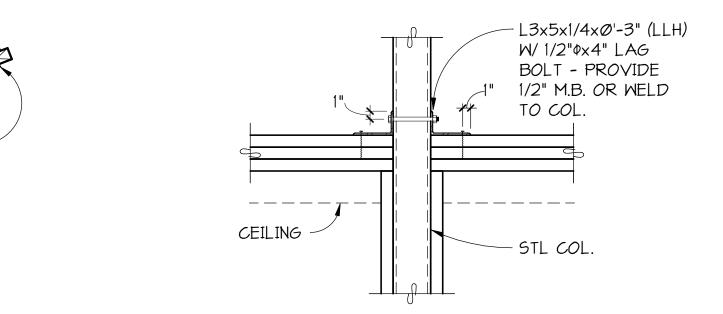
AND INTERSECTIONS.

SECTION

S304 NO SCALE

OTHER LATERAL WALL SUPPORT, CORNERS

TYPICAL TOP OF WALL BRACE UP TO STRUCTURE

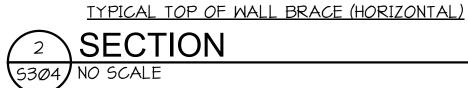


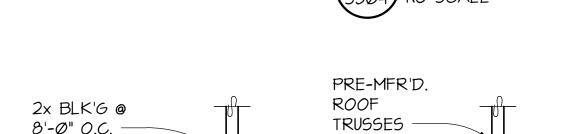
TYPICAL WALL BRACE AT HOLLOW STRUCTURAL STEEL COLUMN

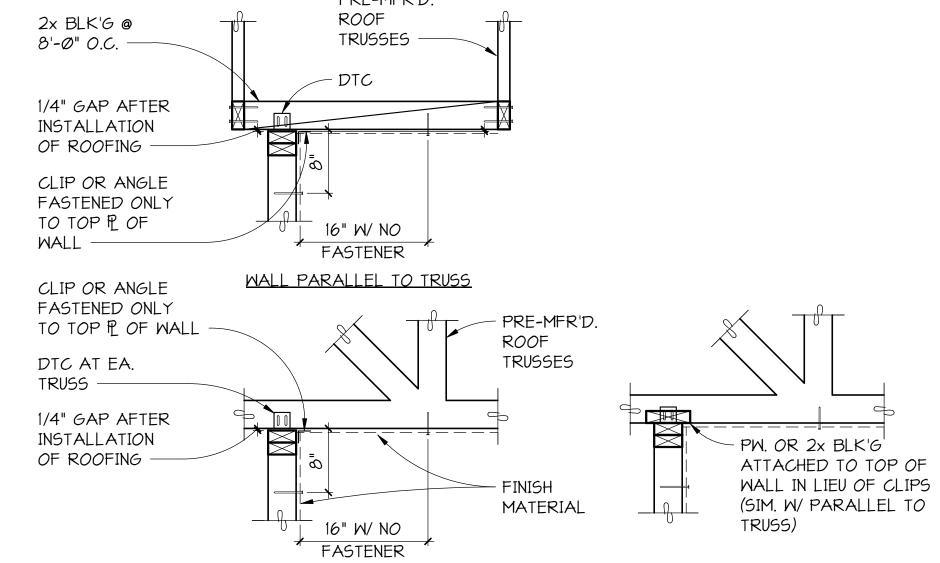
4 SECTION S304 NO SCALE

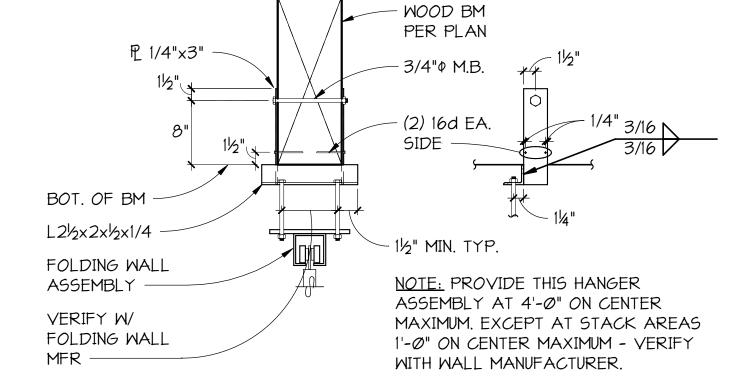
EXTENDING TO STRUCTURE







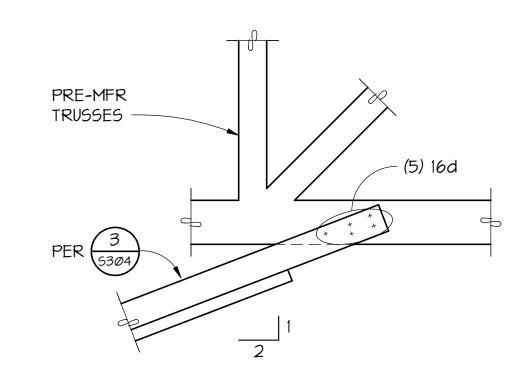




TYPICAL FOLDING WALL SUPPORT AT WOOD BEAM

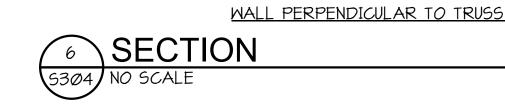


BLOCKING OPTION



TYPICAL BRACE CONNECTION PARALLEL TO TRUSS









Hall

Drawn By:

FRAMING DETAILS

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